Webb County Fairgrounds Master Plan

TRAFFIC STUDY

FINAL REPORT

PREPARED FOR:
Webb County – City of Laredo
Regional Mobility Authority &
HNTB Corporation



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Prepared By: Transcend Engineers & Planners, LLC



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Executive Summary

Transcend Engineers & Planners, LLC (Transcend) is pleased to submit this Traffic Study for the Webb County Fairgrounds Master Plan expansion to Webb County- City of Laredo Regional Mobility Authority (WCCL RMA). The objective of this study was to evaluate traffic impacts to surrounding roadway system, assess ingress/egress operations during various special events, and evaluate internal operations/ circulation for efficiency and safety along US 59 adjoining the Fairgrounds. Operational and safety analysis was performed for four intersections: US 59 at Fairgrounds Entrance 1, US 59 at Fairgrounds Entrance 2, US 59 at Wilson Road, and US 59 at Heritage Court.

Study analysis focused on the following three conditions during a major trip generating event (Concert during a Rodeo). 2.2 occupants per vehicle was assumed to generate trips for Rodeo and Concert. Based on five years of historical TCDS counts and TxDOT Transportation Planning and Programming regression model linear annual growth, a 4.5% growth rate was applied to the existing volumes in order to determine 2025 and 2035 trips.

- I. **Existing Conditions** 2022 traffic conditions with existing lane configuration along US 59 and two existing entrances to the Fairgrounds.
- II. **2025 Build Out Conditions** 2025 traffic conditions with existing lane configuration along US 59 with proposed entrances to the Fairgrounds and trips generated as per the Site Plan.
- III. **2035 Build Out Conditions with US 59/ IH 69 improvements** 2035 traffic conditions with improvements to US 59 as rural divided highway, three lanes in each direction along with projected trips generated by a major event with proposed entrances to the Fairgrounds as per the Site Plan

Operational Analysis

Peak hour data at the Fairgrounds was collected during Laredo International Fair & Exposition Saturday concert event on March 5, 2022. This allowed us to capture the highest possible trips under current event conditions that would enable a comprehensive analysis of the impacts to the surrounding roadway system. 24-hr tube counts on US 59 were collected on Friday, March 4th and Saturday, March 5th, 2022, which helped determine that the peak hours occurred on Saturday from 7:45 to 8:45 PM and 12:00 AM to 1:00 AM (which coincided with concert start and end times, respectively). The study area was analyzed under the following two peak hour scenarios:

- Entry Peak Hour (7:45- 8:45 PM)
- Exit Peak Hour (12:00-1:00 AM)

Lane configurations were developed for both Entry and Exit Peak Hours to accomplish safe and efficient operations for all entrances and US 59. In addition to the proposed lane configurations, two (2) existing entrances for the existing conditions under 2022, three (3) proposed entrances under 2025 and four (4) entrances (3 proposed and 1 existing) under 2035 were taken into consideration.

The following (5) key factors were considered in developing lane configurations as well as ingress and egress designations for 2025 and 2035 Build Out Conditions.

- 1. Minimize conflicting movements
- 2. Maximize safety along US 59
- 3. Alleviate sight distance challenges for Wilson Road and Heritage Court
- 4. Efficient internal circulation
- 5. Effective emergency access

Major Issues Identified from the operational analysis for the different phases are as follows:

Existing Conditions (2022)



- NB left turn movement along US 59 at Existing Entrance 2 is operating at LOS F during Entry Peak
- 0.3 miles queuing was observed along US 59 NB during Entry Peak Hour starting at the Existing Entrance 2.
- During Entry Peak Hour, Sight distance was obstructed for approximately 40 minutes at Wilson Road and 20 minutes at Heritage Court impacting the operations.
- Wb left turns at US 59 and Wilson Road as well as US 59 and Heritage Court intersections are operating at LOS C.

2025 Build Out Conditions

- WB left turn at US 59 and Heritage Court will operate at LOS F during Entry Peak Hour due to high demand and limited gaps in the US 59 SB through traffic.
- EB right-turn at US 59 and Proposed Main Entrance will operate at LOS F during Exit Peak Hour due to limited gaps in the US 59 SB through traffic.

2035 Build Conditions with US 59/IH 69 improvements

- WB left turn at US 59 and Heritage Court will continue to operate at LOS F during both Entry and Exit Peak Hours due to heavy US 59 SB through traffic.
- WB left turn at US 59 and Wilson Road will fail during Exit Peak Hour and operate at unacceptable LOS E during Entry Peak Hour due to heavy US 59 SB through traffic.

Operations at Proposed Main Entrance, Proposed Entrance 3 and Existing Entrance 2 along US 59 improved significantly during both peak hours to LOS D or better under 2035 build conditions due to the proposed improvements.

Recommendations

- Relocate Proposed Main Entrance 2,500 feet away from Wilson Road and designate two Left-Turn lanes during Entry Peak Hour to double the storage capacity and allow vehicles to enter the Fairgrounds quickly to alleviate sight distance challenges for Wilson Road and Heritage Court.
- Provide 4 total lanes for the Proposed Main Entrance with 2 lanes in each direction and an option to implement temporary contraflow conditions depending on the major traffic flow direction.
- Provide an auxiliary lane between Proposed Main Entrance and Existing Entrance 2.
- Utilize Proposed Main Entrance and Existing Entrance 2 for ingress operations and Proposed Entrance 3 for egress during Entry Peak Hour to minimize conflicting movements.
- Utilize Proposed Main Entrance, Proposed Entrance 3, and Existing Entrance 2 for egress operations during Exit Peak Hour, to allow sufficient distance between the exits for the traffic to merge on to US 59 while maximizing safety along US 59.
- Utilize Proposed Entrance 2 for emergency access only during both peak hours.
- Connect Existing Entrance 2 and Proposed Entrance 3 for efficient internal circulation.
- Provide two long driveways on either side of the parking lot, for efficient internal circulation of traffic and storage capacity.
- Utilize various traffic control measures such as staff/security control, traffic cones, flares, directing traffic through staff and signs, for safe and efficient movement of traffic during both peak hours.



1. Project Description and Background

Webb County – City of Laredo Regional Mobility Authority (WCCL RMA) has initiated a traffic study for the Webb County Fairground Master Plan expansion in order to assess traffic impacts and identify mitigation measures as necessary along US 59 adjacent to the Fairgrounds. The Webb County Commissioners Court has been in the process of expanding the Fairgrounds since 2016 to serve a wide range of potential users and activities. As Master Plan and concept site plans are being finalized, the WCCL RMA has engaged the GEC team to evaluate traffic impacts to surrounding roadway system and assess ingress/egress operations during various special events and evaluate internal operations and circulation for efficiency and safety.

Webb County Fairgrounds is located along US 59, east of Laredo City limits. The study area is surrounded by Camp Huisache Aztc District to the north, vacant lands to the east, US 59 to the south, and Webb County Road and Bridge facility to the west. US 59 within study limits has been designated for Interstate conversion to IH 69 and TxDOT is in the process of developing schematic plans for the study corridor. It is expected that US 59 will be a rural divided highway with three lanes in each direction by the year 2035 with improved access for the main entrance to the Fairgrounds.

Fairgrounds are expected to be developed in four (4) Phases. The study assumes Phase 1 to be completed by 2025, phase 2 will be completed by 2035 and US 59 planned improvements to be completed by 2035. Phases 3 and 4 are in conceptual stage without specifics required for study and therefore only phase 1 and phase 2 are included in the analysis. Master Plan also suggested various land uses taking place in the Fairgrounds under various scenarios, however, this study focused on analyzing the land uses that are expected to generate the maximum number of development trips.

2. Study Area

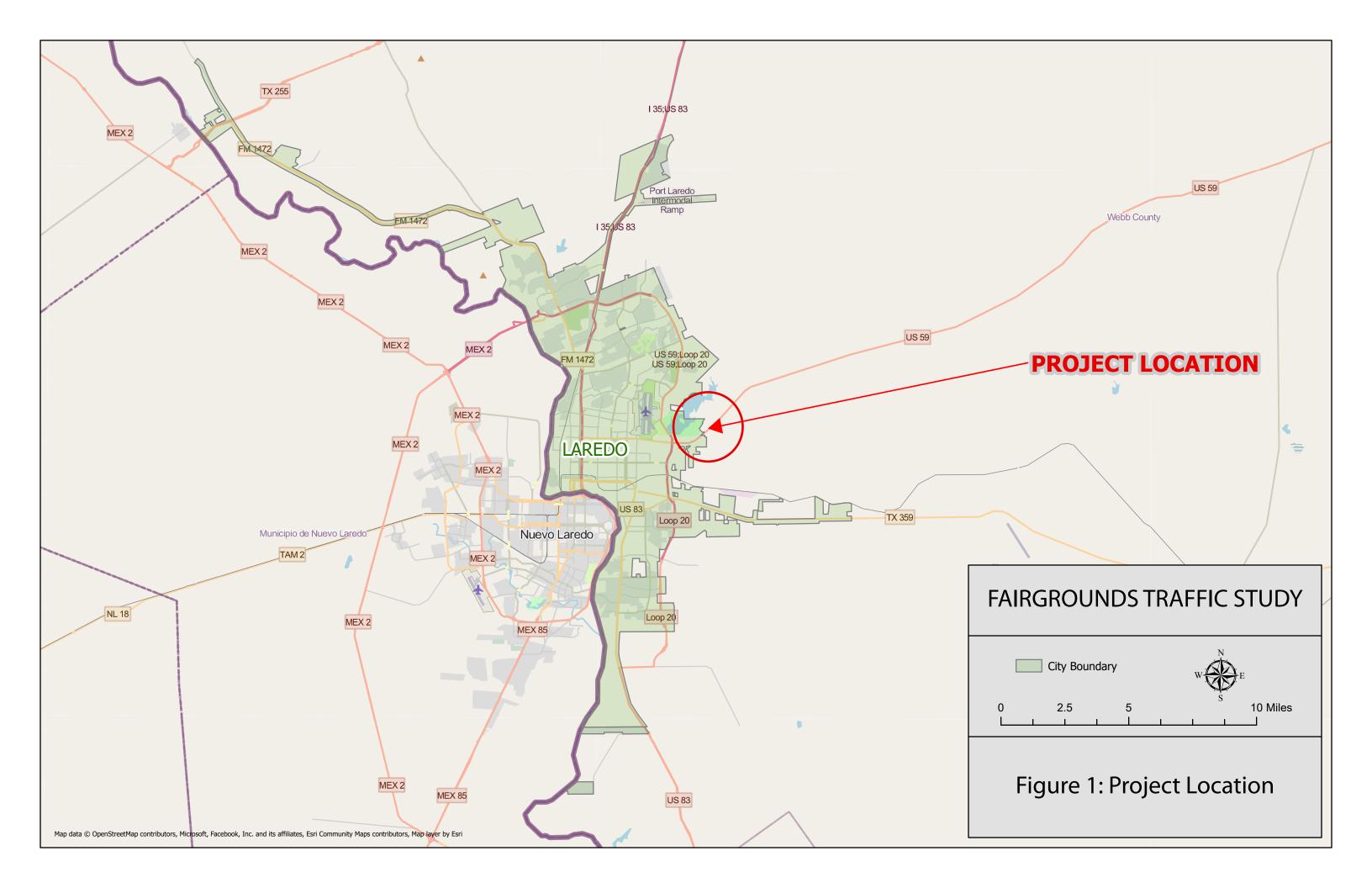
US 59 is the major carrier of traffic volume through the study area which is comprised of institutional and single-family residential uses.

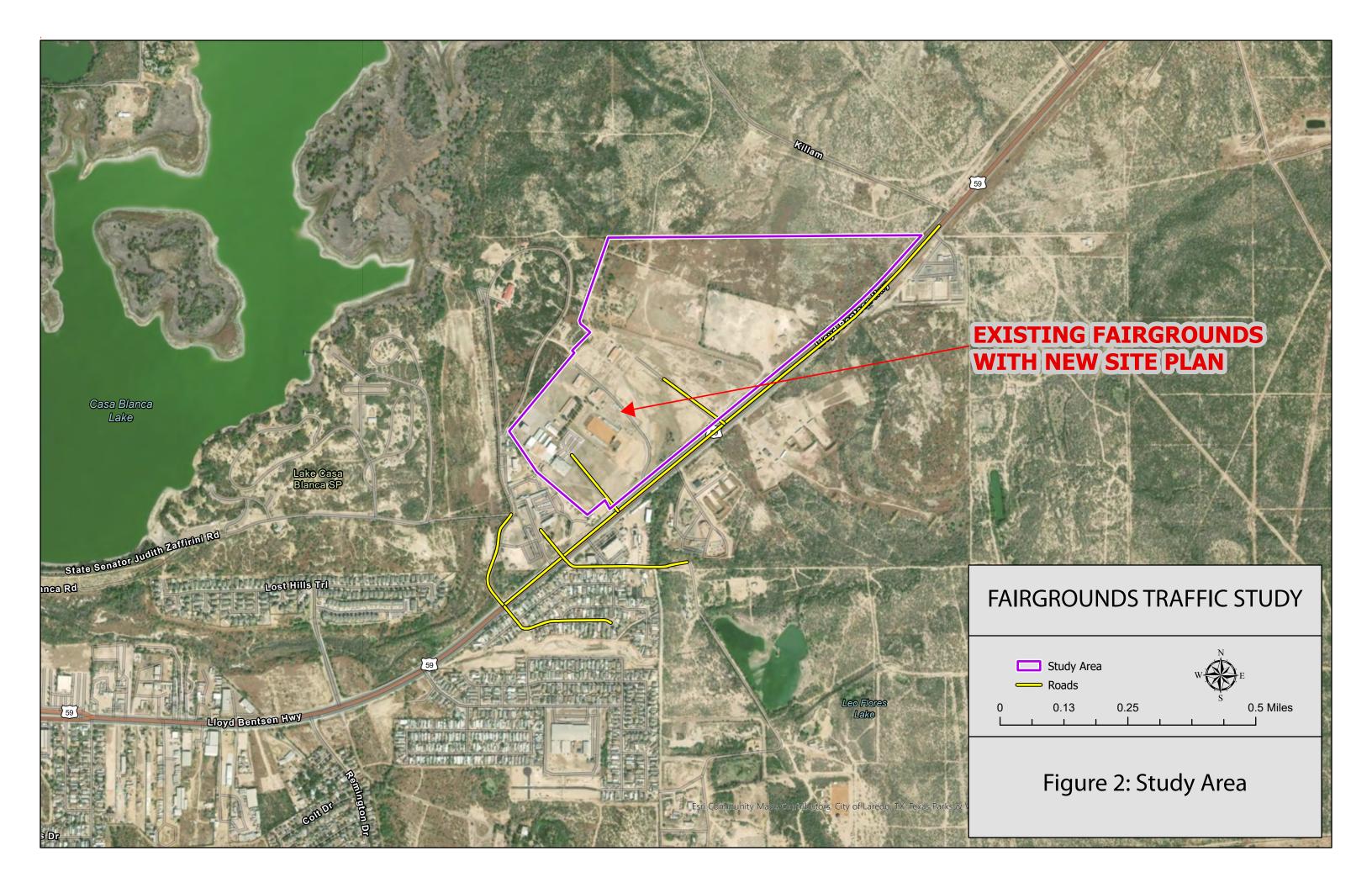
The fairgrounds consists of 140 acres with existing structures that are used for annual county fair, livestock shows, rodeo and riding events with two access points along US 59. Entrance 1, north of the property, is only used during large events when excess parking is required while Entrance 2, south of the property, close to the city limits and is the primary access to the Fairgrounds. Ingress/egress operations at the main entrance to the Fairgrounds is limited to/from City of Laredo to avoid conflicting movements. Webb County Sheriff's department controls the traffic flow during events.

The following four existing intersections are analyzed as part of this traffic study. All intersections in the study area are stop controlled. **Figure 1** shows the project location and **Figure 2** shows the study area.

Study Intersections

- 1. US 59 at Fairgrounds Entrance 1
- 2. US 59 at Fairgrounds Entrance 2
- 3. US 59 at Wilson Road
- 4. US 59 at Heritage Court







3. Methodology

The study scenarios/conditions used for the analysis and an overview of the technical methodology employed to determine Level of Service (LOS) are detailed below.

- 2022 Existing
- 2025 Phase 1 Build Out
- 2035 Phase 1 and Phase 2 Build Out with TxDOT improvements

Traffic and roadway characteristics for the study area roadways and intersections were gathered and incorporated into the microsimulation model (Synchro v10). Road characteristics such as number of lanes, speed limits, and modified travel patterns during Fairgrounds event were obtained from field observations. Queue lengths and traffic volumes were obtained from data collection. US 59 NB segment between Entrance 2 and Heritage Court was evaluated using SimTraffic simulation based on a calibrated Synchro model. Queuing during peak hours was also analyzed for sight distance and safety.

Study intersections were evaluated based on the methodologies outlined in the Highway Capacity Manual, published by the Transportation Research Board. The operating conditions at an intersection were graded by the LOS experienced by drivers. LOS describes the quality of traffic operating conditions and is rated from "A" to "F." LOS A represents the most desirable condition with the free-flow movement of traffic with minimal delays. LOS F indicates severely congested conditions with excessive delays for motorists. Intermediate grades of B, C, D, and E reflect incremental increases in the average delay per stopped vehicle. Delay is measured in seconds per vehicle. **Table 1** shows the upper limit of delay associated with each level of service for unsignalized intersections.

 Level of Service
 Average Vehicle Delay (Seconds)

 A
 ≤ 10

 B
 >10 - 15

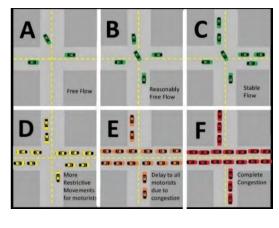
 C
 >15 - 25

 D
 >25 - 35

 E
 >35 - 50

 F
 >50

Table 1. LOS Criteria at Unsignalized Intersections



Note that the HCM 2000 module was utilized for reporting LOS instead of HCM 6 due to the presence of unconventional intersections and missing movements at majority of the study area intersections.



4. Existing Conditions

Lane use and speed limit along US 59 vary within the study limits: two-lane, two-way with a speed limit of 75 mph on the north end of the study area; 4-lane highway with flushed median and a speed limit of 60 mph near Entrance 1; and 6-lane highway with flushed median and speed limit reduced to 50 mph near Entrance 2. Sidewalks are present on both sides of US 59 but marked pedestrian crossings are absent at the study intersections. Bus stops are located along US 59, but none are present within the study area.

Wilson Road is a two-way undivided and unmarked roadway with no posted speed limit signs. The sidewalk is present on one side of Wilson Road but marked pedestrian crossings are absent.

Heritage Court is a two-way undivided roadway with no posted speed limit signs. Sidewalks are present on both sides of Heritage Court but marked pedestrian crossings are absent. Two bus stops are located along Heritage Court.

4.1 Crash Analysis

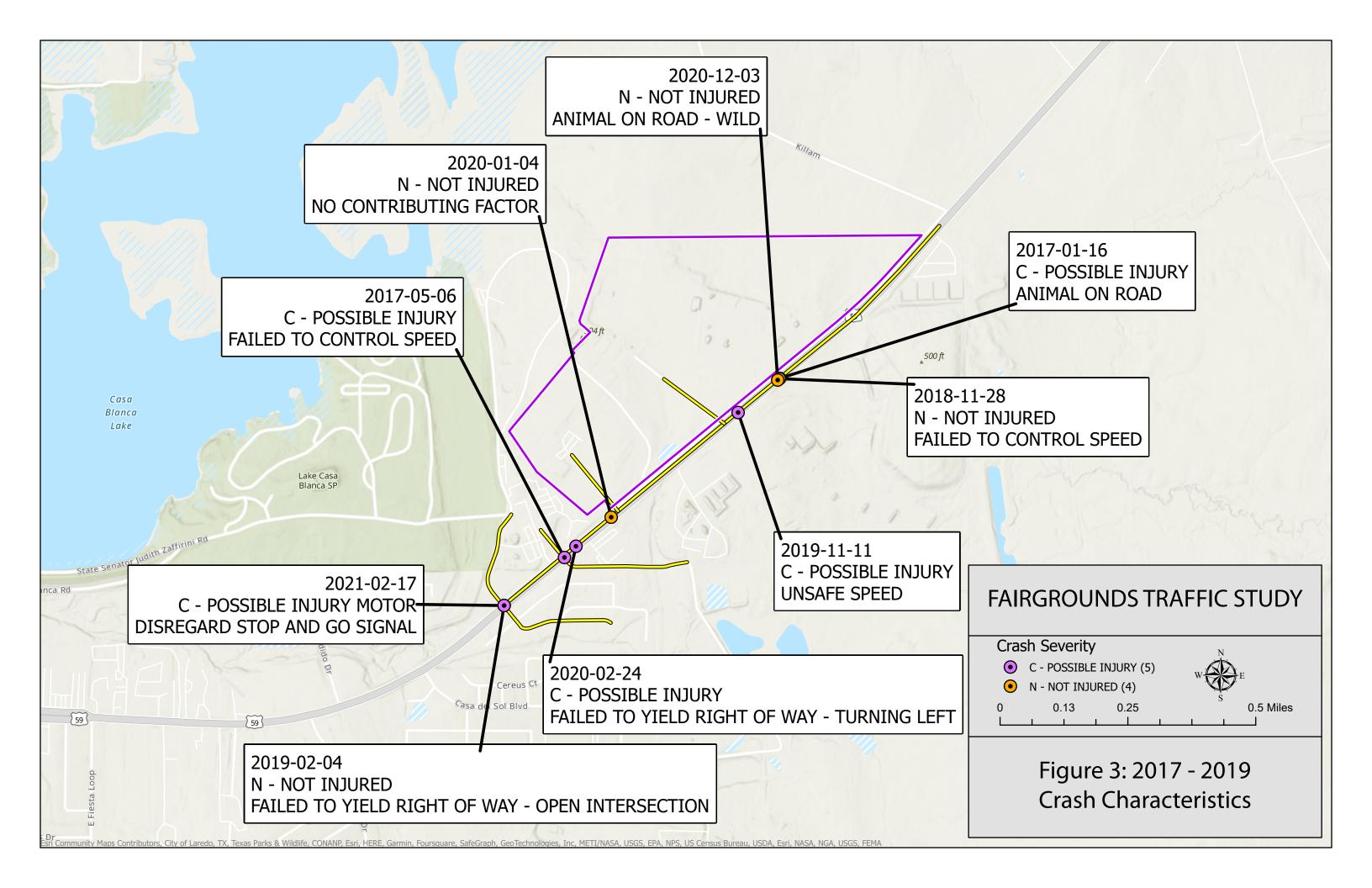
Crash data along US 59 within study limits was obtained from the Crash Records Information System (CRIS) database. A total of 9 crashes occurred between January 2017 and December of 2021 within the study area of which 5 crashes are categorized as possible injury and 4 crashes are not injured. There are zero (0) fatal or serious injury crashes within the study area. Two (2) crashes are intersection related and one (1) crash is driveway access related. Of the total nine (9) crashes, 3 crashes were caused by speed, 2 were caused by wild animals on the road, and 2 failed to yield right of way. Single vehicle going straight is the major crash type with five (5) out of 9 crashes and only one (1) crash occurred during rain and when the surface condition is wet. **Figure 3** shows the crash locations in the study area.

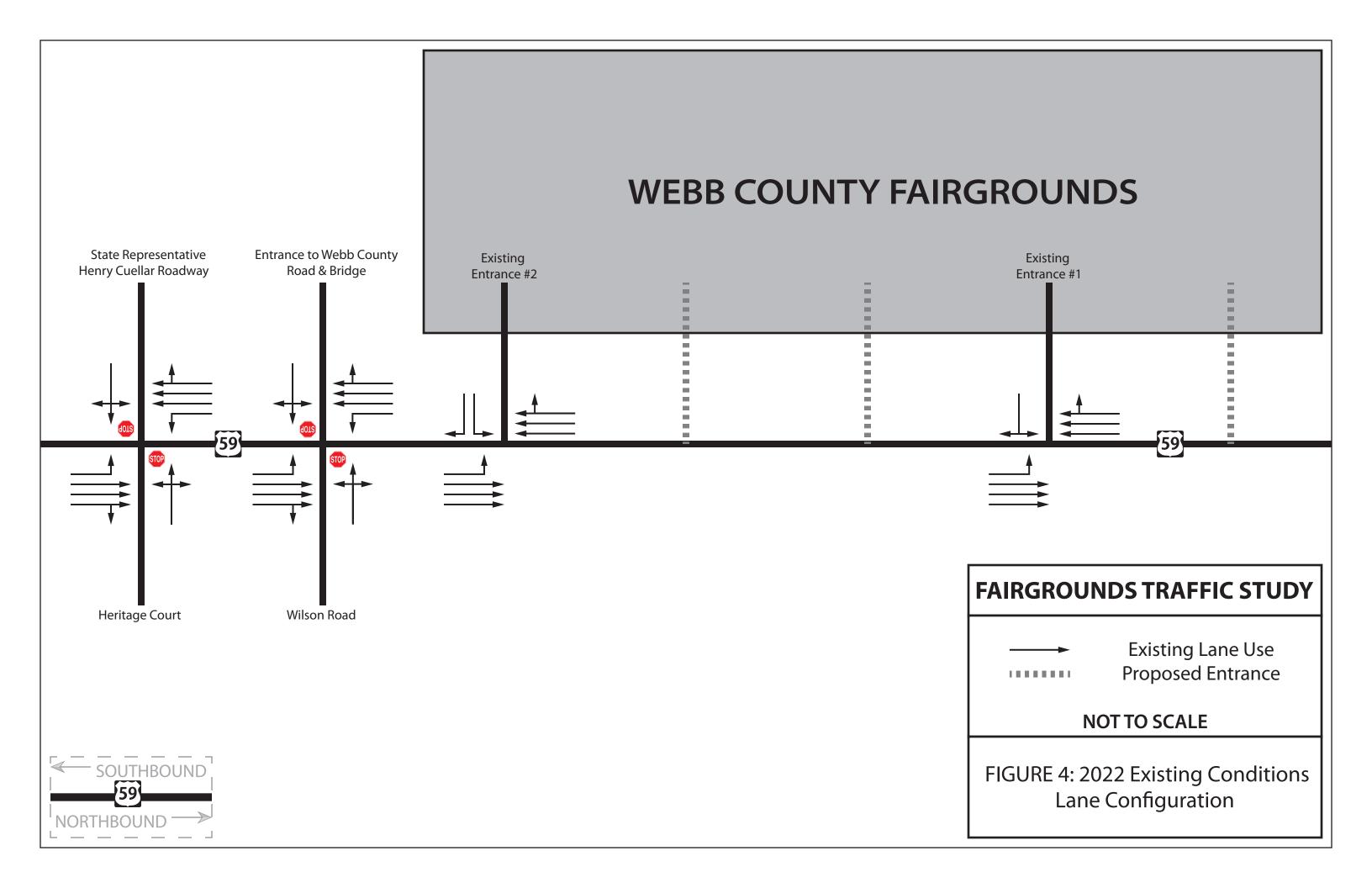
4.2 Data Collection & Traffic Volumes

Laredo International Fair & Exposition (L.I.F.E.) is the largest event held at the Fairgrounds. This week-long fair happens every year at the end of February except during the peak of COVID-19. In 2022, the fair was held from Monday, February 28th to Saturday, March 5th. Saturday Concert was expected to attract most visitors and is considered to create largest backups on US 59. Data was collected during this event to capture the impacts on the surrounding roadway system for comprehensive analysis.

24-hr tube counts on US 59 were collected on Friday, March 4th and Saturday, March 5th, 2022, to determine peak hour volumes. Peak hours occurred on Saturday from 7:45 to 8:45 PM and 12:00 AM to 1:00 AM. These peak hours coincide with concert start and concert end times. In this study, peak hour from 7:45 to 8:45 PM will be referred as Entry Peak Hour and peak hour from 12:00 AM to 1:00 AM as Exit Peak Hour. Turning movement counts for the Entry and Exit peak hours at the study intersections were also obtained for analysis. 2022 Existing Conditions and Lane Configurations for the study area are shown in **Figure 4**.

Raw Traffic Data is shown in Appendix A. Study Area photos are shown in Appendix B.







4.3 Traffic Operations Analysis

During the main event of the fair which includes a concert, traffic patterns are altered to manage the heavy volume and minimize impacts. Webb County Sheriff's Office takes the responsibility of directing incoming and outgoing traffic for the Fairgrounds. Entry to the Fairgrounds is restricted to only Left-Turns from US 59 northbound. In addition, US 59 SB through traffic is altered from free flow to being controlled by marked Police Cars and Officers. Right-turn into the fairgrounds is restricted to eliminate conflicting movements.

It was observed in the field that the Officers were working diligently to the best of their abilities to ensure vehicles enter and exit the Fairgrounds efficiently and safely to the best extent possible. Due to high Entry Peak hour volumes, observed queuing for US 59 northbound left-turns was 0.3 miles through the intersections of Wilson Road and Heritage Court. To calibrate the Synchro model for existing conditions during entry and exit peak hours, a half-signal was added at Entrance 2 to mimic the traffic flow alteration and control by Police Officers.

4.3.1 Level of Service

During Existing Conditions Entry Peak hour, all unsignalized movements are operating at an acceptable LOS C or better expect northbound Left-Turn movement at Entrance 2 which is operating at an unacceptable LOS F. It should be noted that the westbound Left-Turn LOS at both Wilson Road and Heritage Court is impacted by the queuing on US 59 northbound. During Exit Peak Hour, all unsignalized movements are operating at an acceptable LOS C or better.

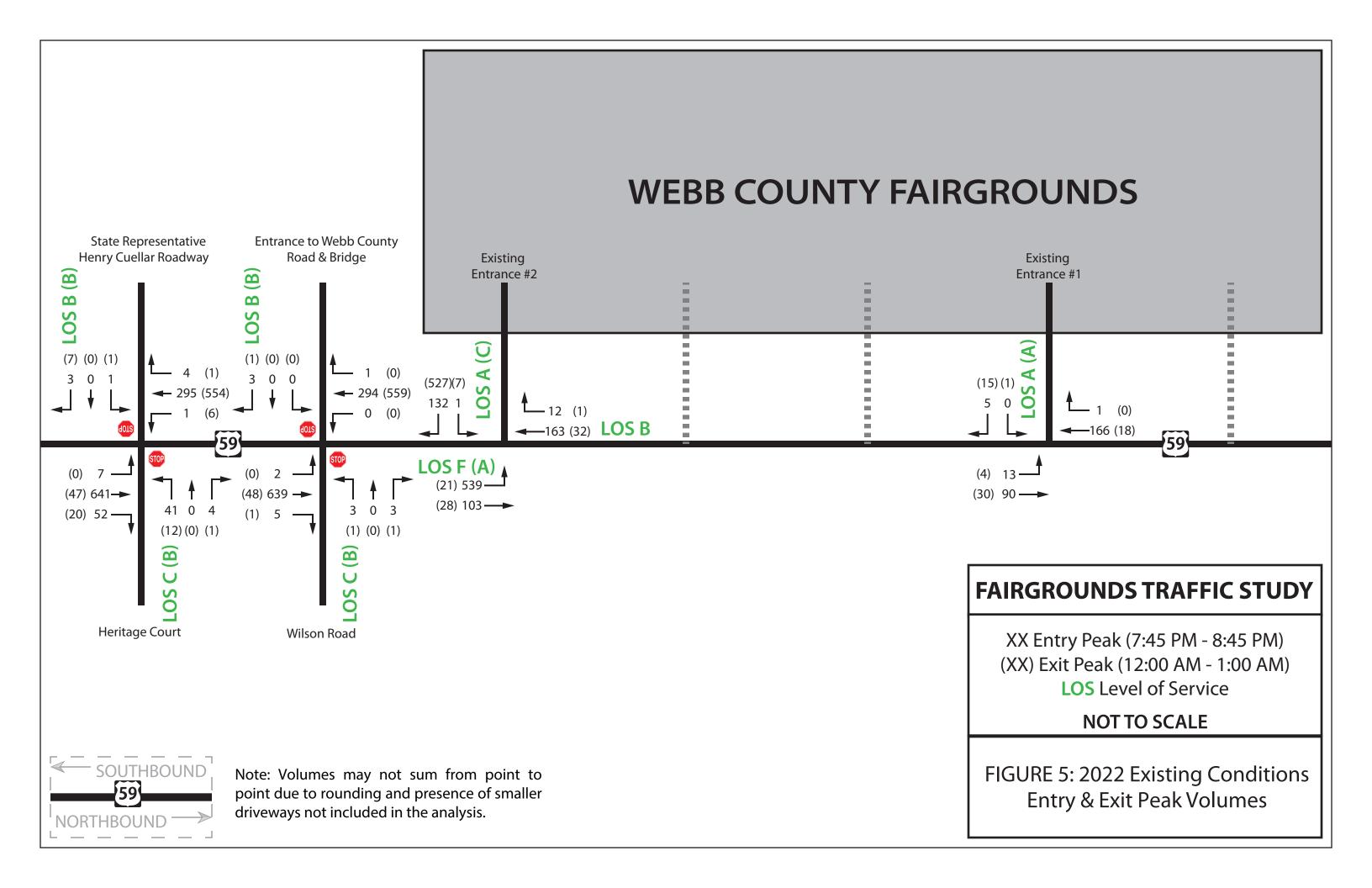
Table 2 summarizes LOS for the study intersections along with Delay and V/C. **Figure 5** shows Entry Peak Hour and Exit Peak Hour volumes.

Synchro Reports for Existing Conditions including delay and v/c ratio are shown in Appendix C.

Table 2. 2022 Existing Conditions LOS

Intersections	E	Entry Peak H	our	Exit Peak Hour		
	LOS	Delay (sec/veh)	V/C	LOS	Delay (sec/veh)	V/C
US 59 at Existing Entrance 1 Eastbound	Α	9.7	0.01	Α	9.2	0.05
US 59 at Existing Entrance 2* Eastbound Northbound Southbound	А F В	5.2 >100 17.7	0.09 > 1 0.23	C A	19.8 8.5 -	0.74 0.03
US 59 at Wilson Road Eastbound Westbound	B C	13.1 15.3	0.03 0.03	B B	14.5 12.2	0.03 0.01
US 59 at Heritage Court Eastbound Westbound	B C	13.4 19.4	0.03 0.18	B B	12.5 11.7	0.05 0.05

Note: LOS and V/C reported are for worst operating approach, *LOS shown is from HCM 2000 module





Longest queue duration: 13 minutes

4.3.2 Queuing and Sight Distance

Given the long queuing along US 59 northbound during Entry Peak Hour, queuing was also analyzed for sight distance and safety for Left-Turns exiting Wilson Road and Heritage Court. Based on the speed limit on US 59, 570 feet of unobstructed sight distance is required to safely make a Left-Turn. In the existing conditions during Entry Peak, sight distance is obstructed for approximately 40 minutes at Wilson Road and approximately 20 minutes at Heritage Court.

SimTraffic Report for Existing Conditions with Queuing and Blocking information is shown in Appendix D.

Table 3 below presents queuing observed on US 59 northbound at Wilson Road and Heritage Court during Entry Peak Hour. Sight Triangles at Wilson Road and Heritage Court are shown in **Appendix E**.

First Queue Time Stamp Queue Length and Number of **Duration of congested Intersection Between 8:00 PM** Vehicles and 9:00 PM US 59 at Wilson Road Distance between Entrance 2 and Total queuing duration Wilson Road is about 700 feet, approximately 40 minutes 7:36 pm equates to 28 vehicles. Longest queue duration: 32 minutes US 59 at Heritage Court Distance between Entrance 2 and Total queuing duration Heritage Court is about 1,530 feet, approximately 20 minutes 8:15 pm equates to 61 vehicles.

Table 3. 2022 Existing Conditions Queuing During Entry Peak

5. Background Growth

Historical TCDS (Traffic Count Database System) counts were used to determine growth trends in the project area. TxDOT's Transportation Planning & Programming regression model for 5 years yields a 4.5% linear annual growth rate. Therefore, 4.5% was added to the existing volumes to determine 2025 Build Conditions and 2035 Build Conditions volumes.

A cross check of ADT along US 59 within the vicinity of study area indicated that the volumes increased at a steady pace even during the COVID-19 pandemic years.

Based on discussion with Webb County, there are no major planned improvements in the immediate vicinity of the study area between now and 2035. Therefore, no additional development traffic is included in the background growth along US 59.

6. Trip Generation & Distribution

6.1 Assumptions

6.1.1 Phasing and Land Use

The proposed Master Plan includes two multipurpose buildings along with horse barns, exhibit hall buildings and show rings. The first multipurpose building is an Arena to accommodate concert, rodeo, sport or equestrian events and Banquet Hall. The second multipurpose building is to host small events such as trade shows, conferences, or ball/dinner. Fairgrounds development is planned in 4 phases. The scope of



this study was limited to the land use(s) that generate more trips than other land uses and to phases that has a build out year within the study period.

Table 4 lists the proposed land uses along with their intensity, phasing, and buildout year.

Table 4. Webb County Fairgrounds Master Plan
Proposed Land Use Phases

Land Use	Intensity	Phase/ Build Out Year
Banquet Hall	20,000 sq.ft.	Phase 1 / 2025
Small Convention	20,000 sq.ft.	Phase 1 / 2025
Rodeo/ Fair	10,000 People	Phase 1 / 2025
Concert	4,500 Seats	Phase 1 / 2025
Convert existing banquet hall to office space	20,000 sq.ft.	Phase 2 / 2035
Outdoor Amphitheater	Intensity unknown at this time	Phase 3 / Likely beyond 2035
Hotel	Intensity unknown at this time	Phase 4 / Likely beyond 2035

Based on existing conditions and the type of proposed development, non-event trips generated at any of the study area intersections should not warrant additional traffic control.

6.1.2 Rodeo & Concert Trips

Among the five land uses, Rodeo and Concert are the major trip generators with both of them taking place on the same day during the Fair. It should be noted that there are no standard trip generation tables available for these two land uses in ITE Trip Generation manual. Based on relevant research, it is assumed that both Rodeo and Concert result in 2.2 occupants per vehicle.

For event-based land uses, the majority of traffic enters at the start of the event and exits at the end of the event. Therefore, two distributions were analyzed i.e., Entry Peak Hour and Exit Peak Hour. Phase 2 land use, Office, will not have any trips generated on a Saturday evening when the Concert happens.

Rodeo Trips: Applying 2.2 occupancy per vehicle for 10,000 attendees results in 4,545 trips. Given that Entry Peak Hour is the end of the Rodeo, only exit trips from the Rodeo are considered for this analysis. Based on research and existing conditions, it is estimated that 10% of the total Rodeo trips (455 trips) will exit.

Concert Trips: Applying 2.2 occupancy per vehicle for 4,500 attendees results in 2,045 trips. An estimated 25% (511 trips) internal trip reduction is applied for Entry Peak Hour trips. No internal trip reduction is applied to Exit Peak Hour trips.

6.2 Trip Generation

Table 5 on the next page presents the Entry Peak Hour and Exit Peak Hour trip generation for Rodeo and Concert based on the land use and trip assumptions detailed above.



Table 5. Trip Generation

Land Use Intensity				Event	Entry Peak Hour					Exit Peak Hour				
		Trip Rate	Total	% In	% Out	ln	Out	Total	% In	% Out	In	Out		
Rodeo	(Phase 1)	10,000	people	0.45	4,545		10%		455	0	0%	100%		0
Concer	t (Phase 1)	4,500	seats	0.45	2,045	80%	20%	1,636		4,545	0%	100%	0	4,545
Office	(Phase 2)	20,000	Sq.ft.		0	0%	0%	0	0	0	0%	0%	0	0
Office	(Phase 2)	20,000	Sq.ft.		0	0%	0%	0	0	0	0%	0%	0	0
Internal Trips between Rodeo and Concert 25%					(511)	0	0			0	0			
Concert Trips after Internal Trip Reduction		1,534	80%	20%	1,125		4,545	0%	100%	0	4,545			
Total Future Phase Only Development Trips added to Adjacent Street			1,682			1,125	455	4,545			0	4,545		

6.3 Trip Distribution

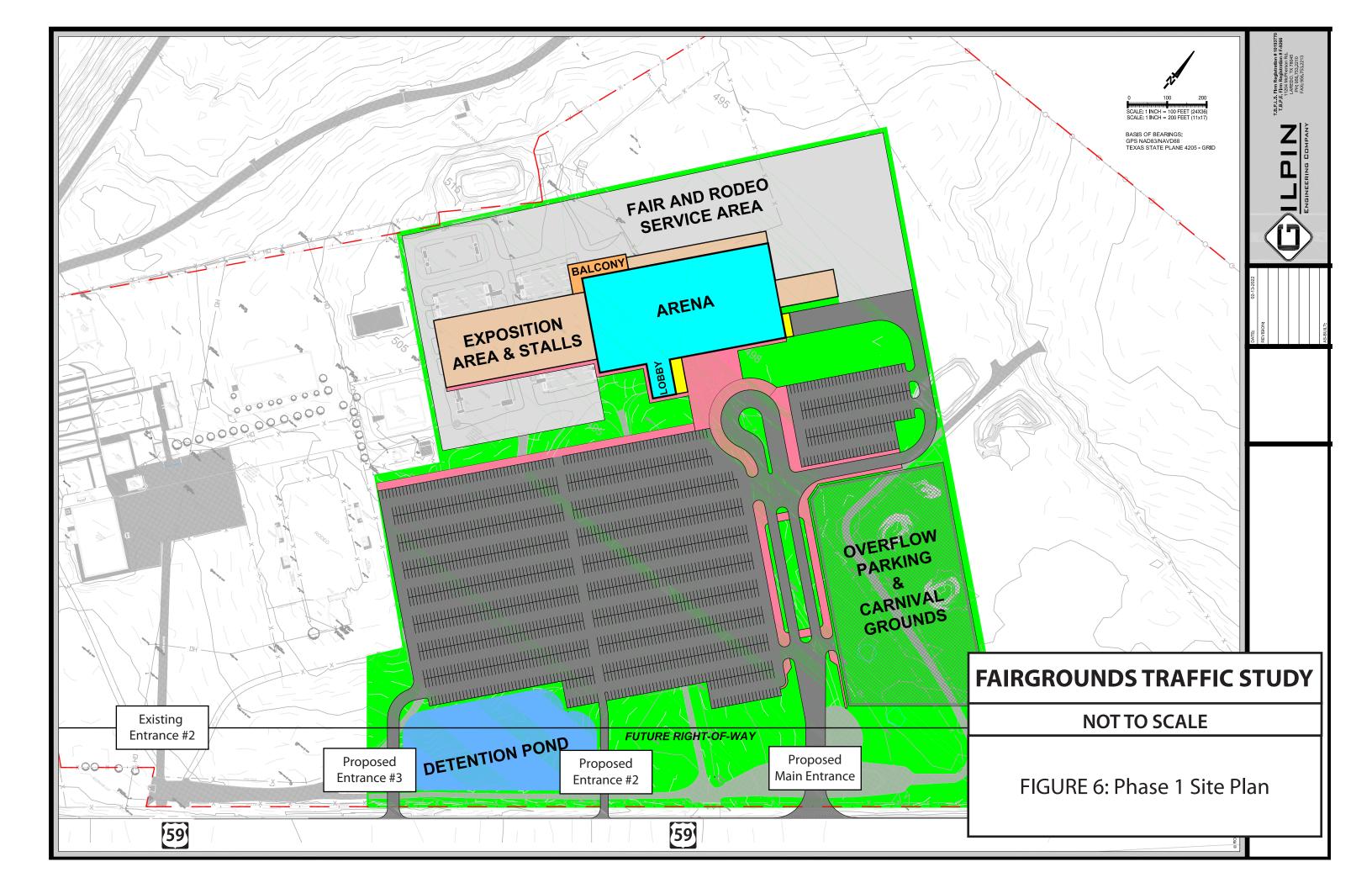
Based on existing traffic patterns and assumptions described in Section 6.1, 90% of the trips are distributed to/from US 59 northbound and 10% trips to/from US 59 SB.

Furthermore, to achieve comprehensive and effective distribution of trips during both Entry and Exit Peak Hours that would result not only in safe/quick ingress/egress to the Fairgrounds but also maintain safe operations along US 59, the following key factors were considered:

- 6. Minimize conflicting movements
- 7. Maximize safety along US 59
- 8. Alleviate sight distance challenges for Wilson Road and Heritage Court
- 9. Efficient internal circulation
- 10. Effective emergency access

7 2025 Build Conditions

The proposed Phase 1 Site Plan includes a total of four entrances to the Fairgrounds (three proposed and one existing). All three proposed entrances are north of current Entrance 2. **Figure 6** shows the Proposed Phase 1 Site Plan.





7.1 Site Access

7.1.1 Lane Configuration

Lane configurations were developed for both Entry and Exit Peak Hours to accomplish safe and efficient operations for all entrances and US 59 and satisfy the five key factors mentioned in section 6.3.

- 1. Proposed Main Entrance and Proposed Entrance 3 will be utilized for ingress while Proposed Entrance 2 will be utilized for egress during Entry Peak Hour to **minimize conflicting movements**.
- Designating Proposed Main Entrance and Proposed Entrance 3 for exit trips during Exit Peak Hour will allow sufficient distance between the exits for traffic to merge on to US 59 to maximize safety along US 59.
- 3. Proposing Main Entrance 2,500 feet away from Wilson Road and designating two Left-Turn lanes during Entry Peak Hour doubles the storage capacity and allow vehicles to enter the Fairgrounds quickly will **alleviate sight distance challenges** for Wilson Road and Heritage Court.
- 4. Two long driveways on either side of the parking lot will facilitate **efficient internal circulation** and will provide storage when necessary, during Entry and Exit Peak Hours.
- 5. During Exit Peak Hour, Proposed Entrance 2 will operate as **emergency access** to disperse traffic efficiently and as exclusive access to EMS vehicles during emergencies.

7.1.2 Ingress and egress designations

The following ingress and egress designations were developed to account for the key considerations above:

- Proposed Main Entrance is assigned 60% of the Entry Peak Hour ingress trips and 50% of Exit Peak Hour egress trips. Of the 60% ingress trips, 50% are Left-Turns and 10% are Right-Turns.
- Proposed Entrance 2 is designated for all egress trips during Entry Peak Hour and no trips during Exit Peak Hour. In addition, Proposed Entrance 2 will be utilized for emergency access.
- Proposed Entrance 3 is designated to receive 40% of the Entry Peak Hour ingress trips and 35% of Exit Peak Hour egress trips.

Entry Peak hour

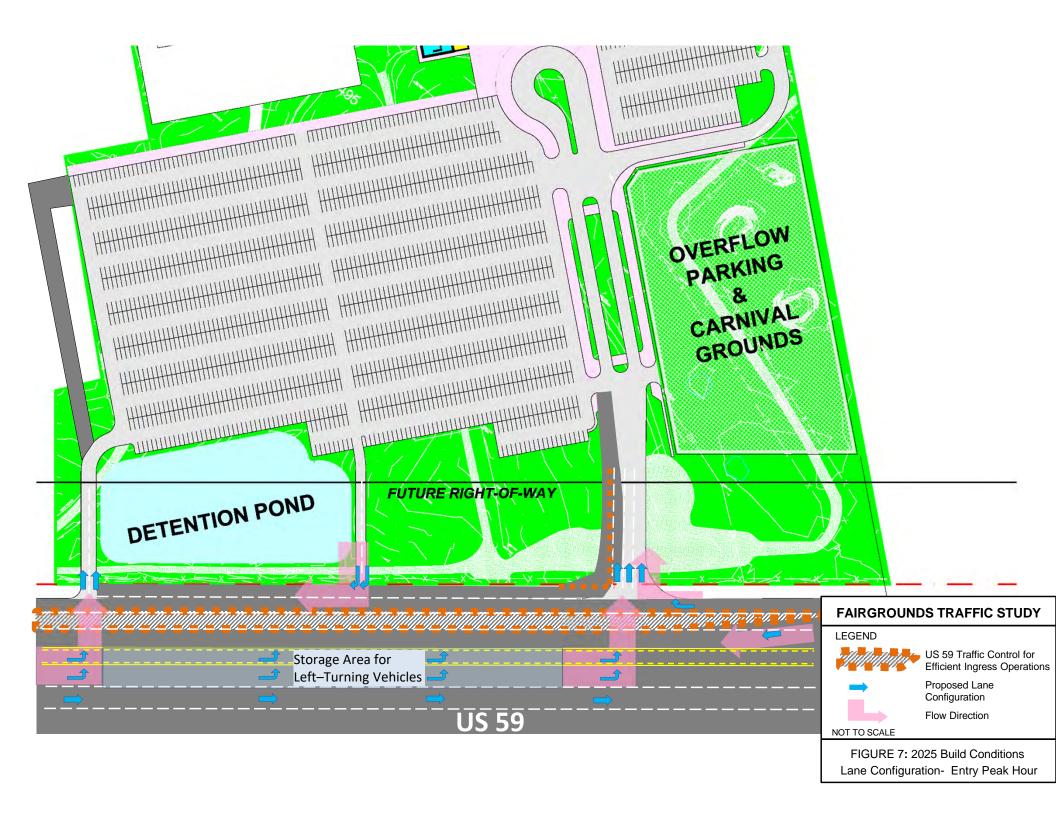
- US 59 northbound: reconfigured to one-lane between Wilson Road and Proposed Main Entrance and two left-turn lanes between Proposed Main Entrance and south of Proposed Entrance 3.
- Proposed Main Entrance: three lanes for ingress (two-lanes for northbound Left-Turns and onelane for southbound Right-Turns)
- Proposed Entrance 2: eastbound turn right only for egress to avoid conflicting movements.
- Proposed Entrance 3: two northbound Left-Turn only lanes for ingress and southbound Right-Turns prohibited.

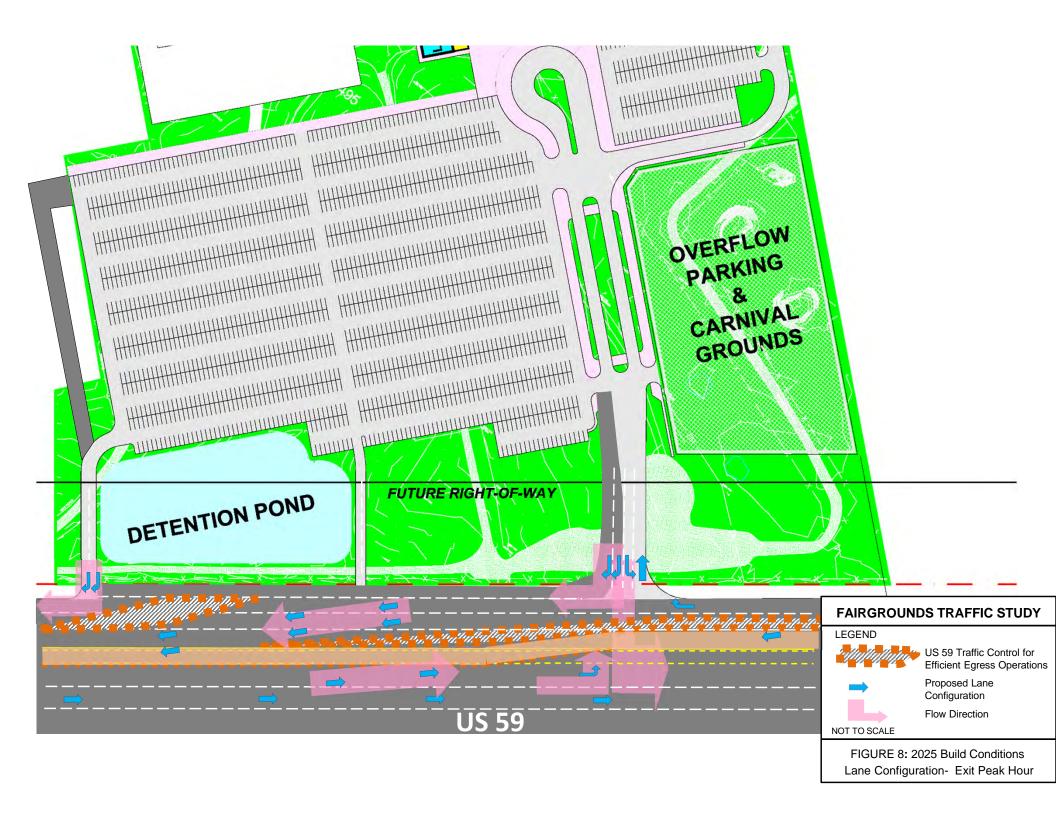
Figure 7 shows the lane configuration and ingress/egress operations for Entry Peak Hour.

Exit Peak hour

- US 59 SB: reconfigured to one-lane north of Proposed Main Entrance to accommodate safe/efficient egress for all entrances.
- Proposed Main Entrance: three lanes for egress (two-lanes for eastbound Right-Turns and one-lane for eastbound Left-Turn) and one-lane for ingress
- Proposed Entrance 2: no ingress/egress; emergency access only.
- Proposed Entrance 3: two eastbound turn right only lanes for egress.

Figure 8 shows the lane configuration and ingress/egress operations for Exit Peak Hour.







7.2 Traffic Operations Analysis

During 2025 Build Conditions Exit Peak, eastbound Right-Turn movement at US 59 and Proposed Main Entrance operates at unacceptable LOS F due to excessive demand. During 2025 Build Conditions Entry Peak, westbound left-turn movement at US 59 and Heritage Court operates at unacceptable LOS F due to limited gaps in the heavy US 59 SB through traffic. All remaining unsignalized movements at the study intersections are operating at an acceptable LOS D or better.

Table 6 summarizes LOS for the study intersections along with Delay and V/C. **Figure 9** shows Entry Peak Hour and Exit Peak Hour volumes under 2025 Build Conditions.

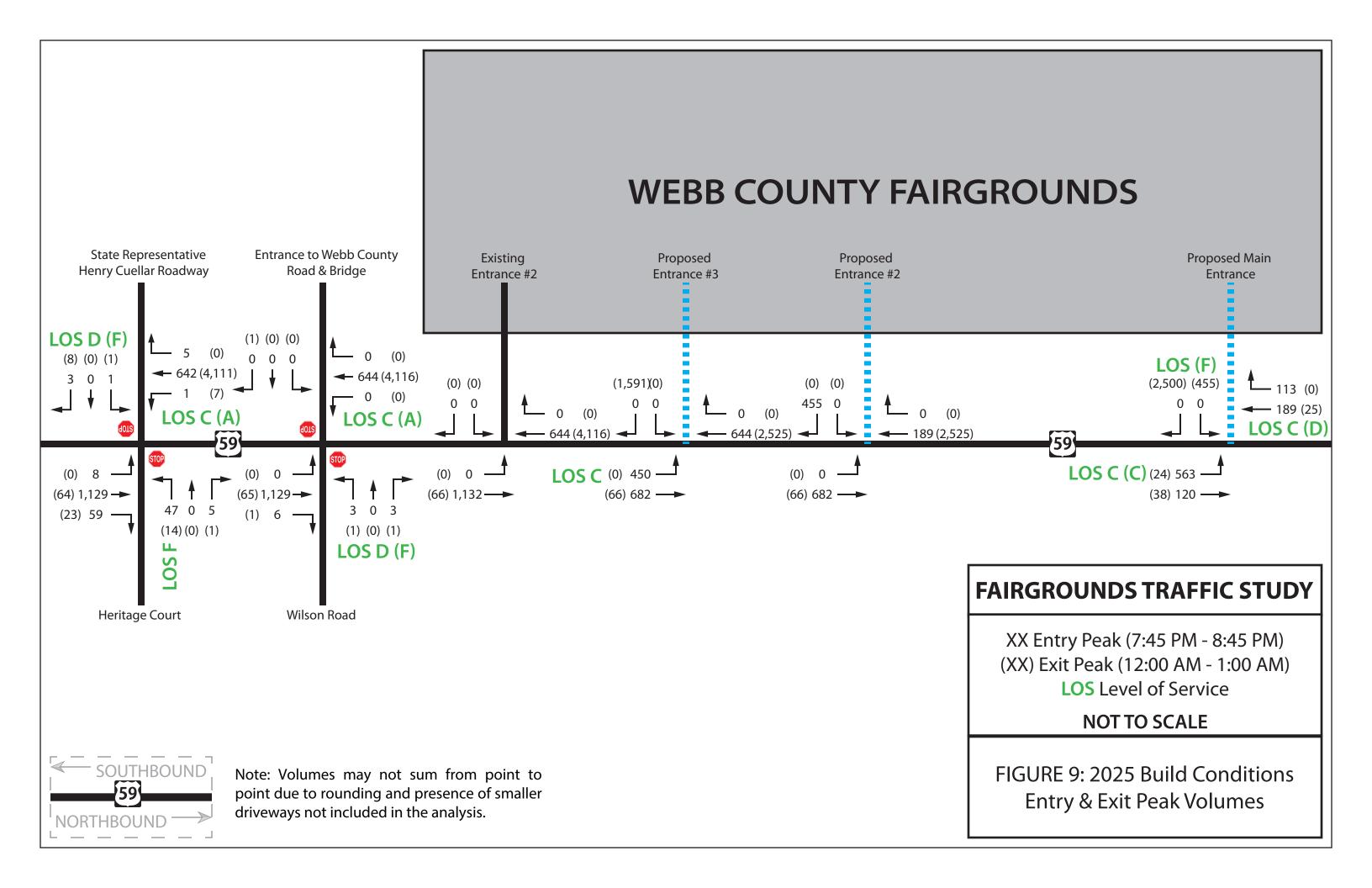
Synchro Reports for 2025 Build Conditions including delay and v/c ratio are shown in Appendix F.

Table 6. 2025 Build Condition LOS

Intersections	Entr	y Peak Hour		Exit Peak Hour			
	LOS	LOS Delay V/C (sec/veh)			Delay (sec/veh)	V/C	
US 59 at Proposed Main Entrance* Northbound Eastbound	С	25.5	0.74	F	89.2	>1	
US 59 at Proposed Entrance 2	Free		Emergency Access Only				
US 59 at Proposed Entrance 3* Northbound	С	17.2	0.66	Free flowing traffic			
US 59 at Wilson Road Westbound Southbound	D C	32.3 17.1	0.07 0.00	А	8.5	0.00	
US 59 at Heritage Court Westbound Southbound	F C	65.8 17.7	0.53 0.00	A	8.5	0.01	

Note: LOS and V/C reported are for worst operating approach, *LOS shown is from HCM 2000 module

SimTraffic Report for 2025 Build Conditions with Queuing and Blocking information is shown in Appendix G.





8 2035 Build Conditions with US 59/IH 69 improvements

Phase 2, conversion of existing banquet hall to office space, is expected to be completed before 2035. However, this land use will not generate any trips during Entry Peak Hour and Exit Peak Hour as mentioned in section 6.1.2. Henceforth, 2035 Build Condition is similar to 2025 Build Conditions except background growth.

As part of US 59 conversion to IH 69, TxDOT is developing a schematic that includes the study limits. During project scoping, TxDOT clarified that under 2035 conditions, US 59 should be considered as a rural divided highway with three lanes in each direction with improved access at the Proposed Main Entrance. As part of this traffic study, grade access is proposed in order to provide TxDOT the flexibility to evaluate the best long-term solution. It should be noted that TxDOT is currently implementing a systemic raised median project district wide which includes the study segment of US 59.

8.1 Site Access

8.1.1 Lane Configuration

2035 Lane configurations were developed with US 59 TxDOT suggested improvements for both Entry and Exit Peak Hours that satisfies the five key factors mentioned in section 6.3.

- 1. Proposed Main Entrance and Existing Entrance #2 will be utilized for ingress while Proposed Entrance 3 will be utilized for egress during Entry Peak Hour to **minimize conflicting movements**.
- Designating Proposed Main Entrance and Existing Entrance #2 for egress during Exit Peak Hour will allow sufficient distance between the exits for traffic to merge on to US 59 to maximize safety along US 59.
- Relocated Main Entrance 2,500 feet away from Wilson Road and designated two Left-Turn lanes
 to alleviate sight distance challenges for Wilson Road and Heritage Court from 2025 Build
 Conditions also applies to 2035 Build Conditions.
- 4. Two long driveways on either side of the parking lot to facilitate **efficient internal circulation** from 2025 Build Conditions scenario also applies to 2035 Build Conditions.
- 5. During both Peak Hours, Proposed Entrance 2 will operate as **emergency access** to disperse traffic efficiently and as exclusive access to EMS vehicles during emergencies.

8.1.2 Ingress and egress designations

The following ingress and egress designations were developed to account for the key considerations above:

- Proposed Main Entrance is assigned 60% of the Entry Peak Hour ingress trips and 45% of Exit Peak Hour egress trips. Of the 60% ingress trips, 50% are Left-Turns and 10% are Right-Turns. Of the 45% of the egress trips, 35% are Right-Turns and 10% are Left-Turns.
- Proposed Entrance 2 will be utilized for emergency access only.
- Proposed Entrance 3 is designated for all egress trips during Entry Peak Hour and 20% egress trips during Exit Peak Hour. Egress trips at this entrance is only by Right-Turns.
- Existing Entrance 2 is designated to receive 40% of the Entry Peak Hour ingress trips by Left-Turns. 35% of Exit Peak Hour egress trips are Right-Turns only.

Entry Peak hour

- US 59 northbound: reconfigured to one-lane between Wilson Road and Proposed Main Entrance and two left-turn lanes between Proposed Main Entrance and south of Proposed Entrance 3.
- Proposed Main Entrance: three lanes for ingress (two-lanes for northbound Left-Turns and onelane for southbound Right-Turns).



- Proposed Entrance 2: no ingress/egress; emergency access only.
- Proposed Entrance 3: two southbound Right-Turn only lanes for egress.
- Existing Entrance 2: two-lanes for ingress and northbound Left-Turn only.

Figure 10 shows lane configuration and ingress/egress operations for 2035 Build Conditions Entry Peak.

Exit Peak hour

- US 59 SB: reconfigured to one-lane north of Proposed Main Entrance to accommodate safe/efficient egress for all entrances.
- Proposed Main Entrance: three lanes for egress (two-lanes for eastbound Right-Turns and one-lane for eastbound Left-Turn) and one-lane for ingress.
- Proposed Entrance 2: no ingress/egress; emergency access only.
- Proposed Entrance 3: one eastbound turn right only lanes for egress.
- Existing Entrance 2: one eastbound turn right only lanes for egress.

Figure 11 shows lane configuration and ingress/egress operations for 2035 Build Conditions Exit Peak.

8.2 Traffic Operations Analysis

During 2035 Build Conditions, US 59 at the Proposed Main Entrance and US 59 at the Existing Entrance 2, where the primary access to the Fairgrounds is proposed, are operating at acceptable LOS D or better during Entry and Exit Peak Hours. At Proposed Entrance 3, traffic is free flowing during both peak hours. During both Entry and Exit Peak conditions, westbound left-turn movement at US 59 and Heritage Court is failing due to limited gaps in the heavy US 59 through traffic. During Exit Peak Hour, westbound left-turn movement at US 59 and Wilson Road is also failing due to heavy US 59 through traffic.

Table 7 summarizes LOS for the study intersections along with Delay and V/C. **Figure 12** shows Entry Peak Hour and Exit Peak Hour volumes under 2035 Build Conditions.

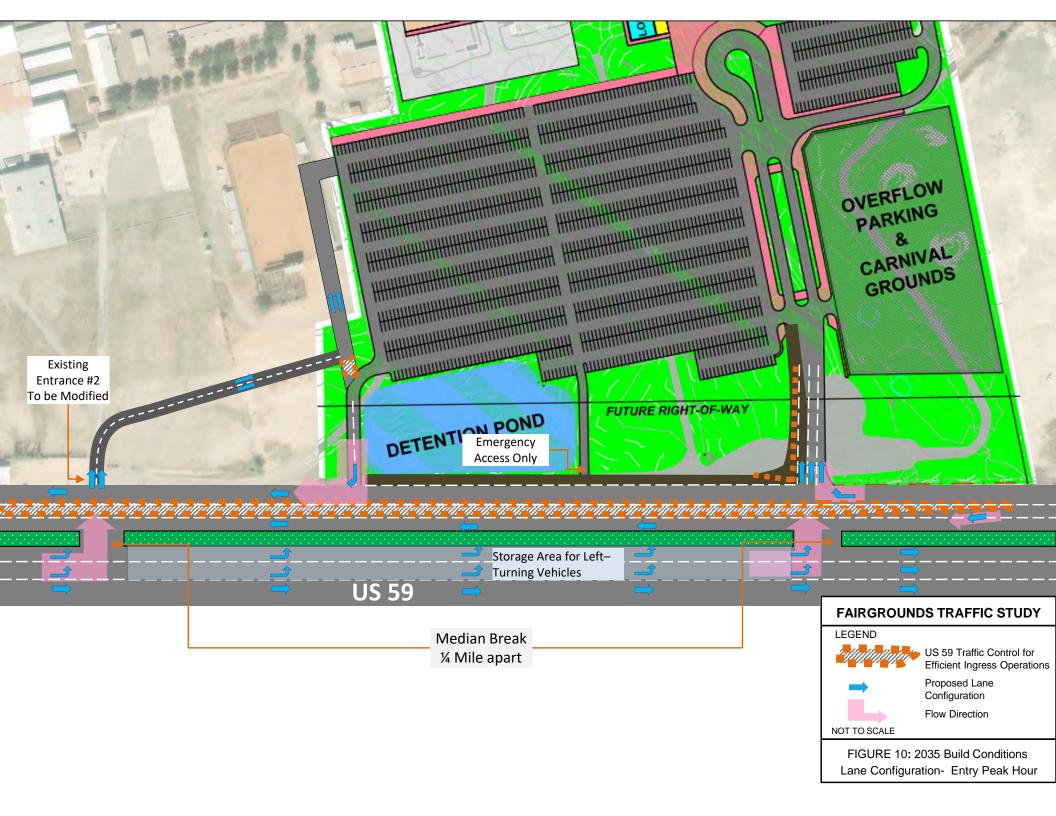
Synchro Reports for 2035 Build Conditions including delay and v/c ratio are shown in Appendix HF.

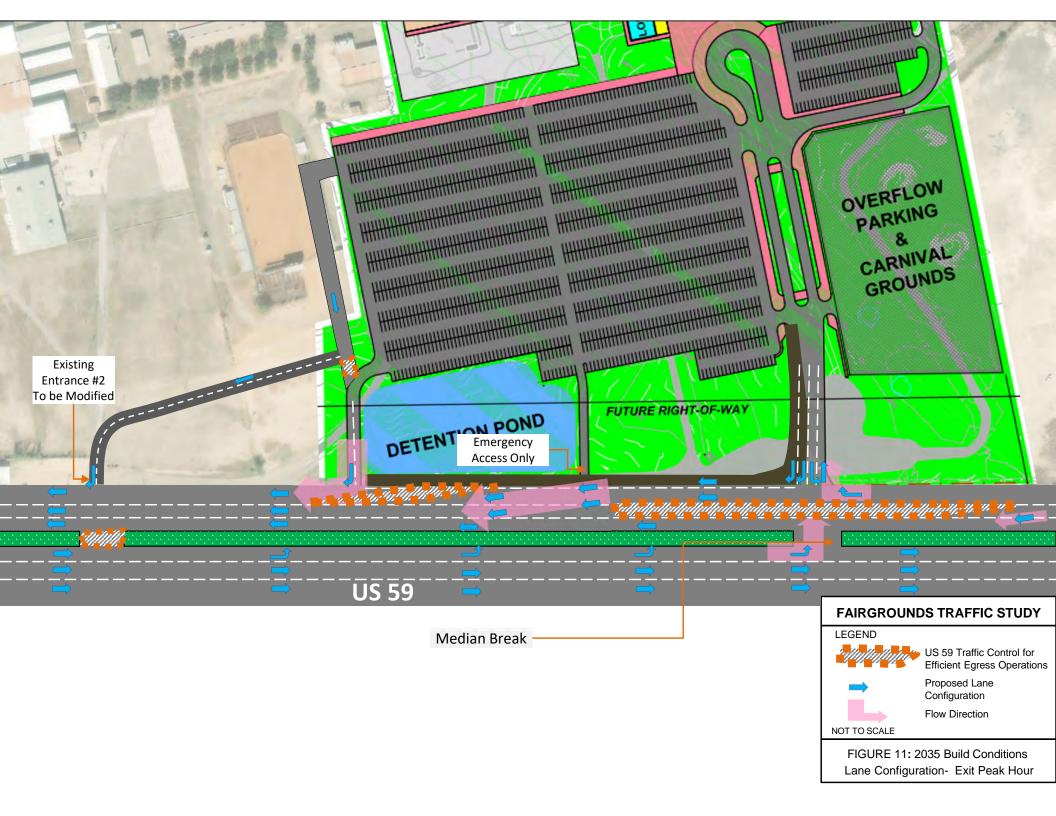
SimTraffic Report for 2025 Build Conditions with Queuing and Blocking information is shown in Appendix G.

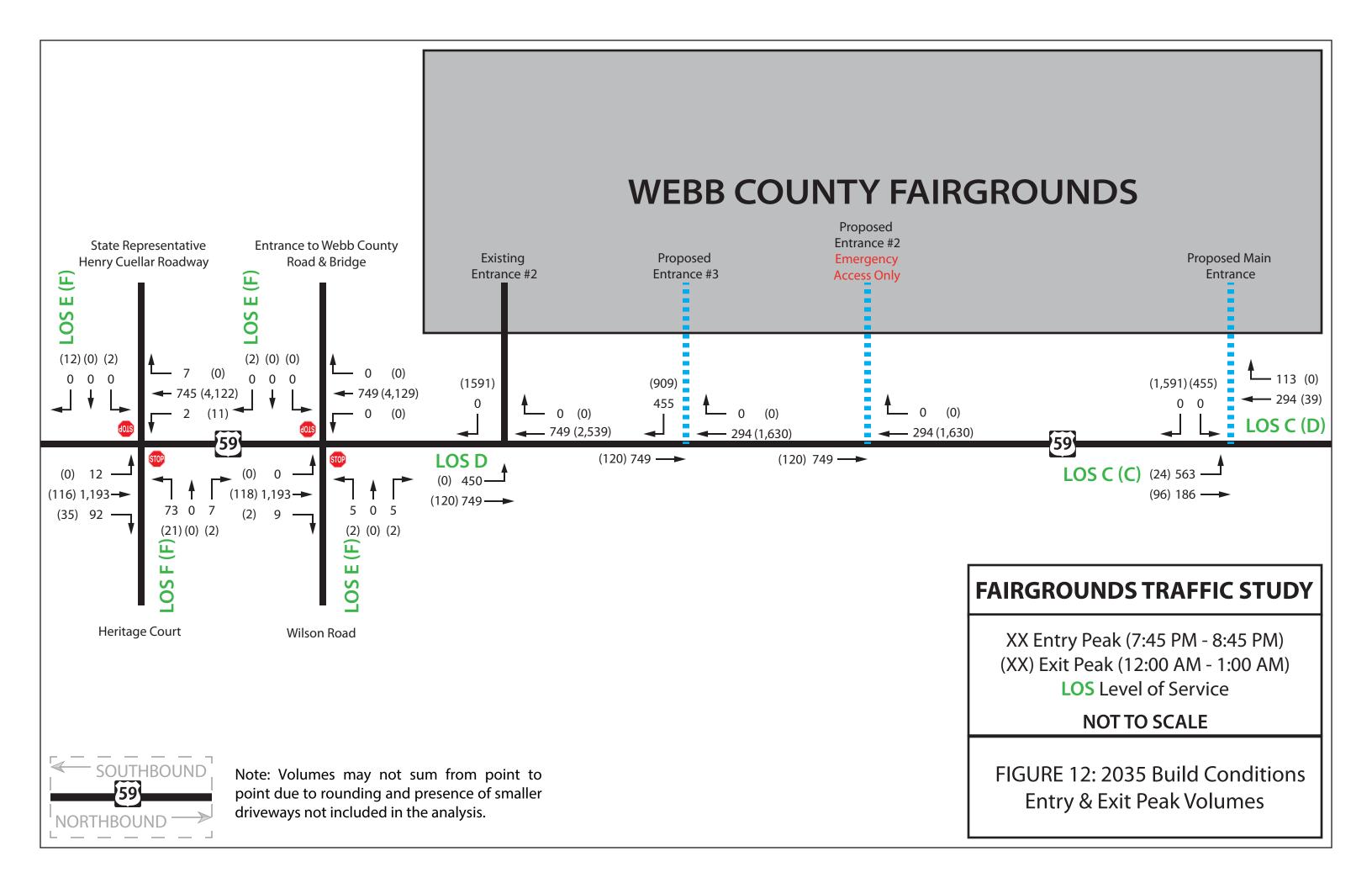
Table 7. 2035 Build Condition LOS

Intersections	E	ntry Peak Ho	our	Exit Peak Hour			
	LOS	Delay (sec/veh)	V/C	LOS	Delay (sec/veh)	V/C	
US 59 at Proposed Main Entrance*							
Southbound	С	32.7	0.78	D	0.20		
US 59 at Proposed Entrance 2	Emer	gency Acces	s Only	Emergency Access Only			
US 59 at Proposed Entrance 3*	Fr	ee flowing tra	affic	Free flowing traffic			
US 59 at Existing Entrance 2* Northbound	D	38.4	0.86	Free flowing traffic			
US 59 at Wilson Road Westbound	E	36.7	0.12	F >100 0.39			
US 59 at Heritage Court Westbound	F	>100	1.00	F	>1		

Note: LOS and V/C reported are for worst operating approach, *LOS shown is from HCM 2000 module









9. Evaluation of Final Site Plan for Internal Operations and Circulations

Internal circulations for the proposed site plan was developed that satisfies the five key factors mentioned in section 6.3 and US 59 TxDOT suggested improvements for both Entry and Exit Peak Hours. Following traffic control measures will be utilized for safe/quick ingress/egress to the Fairgrounds:

- Staff/security control of inbound and outbound traffic from/to US 59
- Lane revisions/enhancements using traffic cones and flares to temporarily "add/remove" lanes
- Staff/security directing to/from parking lots
- Signs to direct traffic to/from parking areas

9.1 Entry Peak Hour Operations

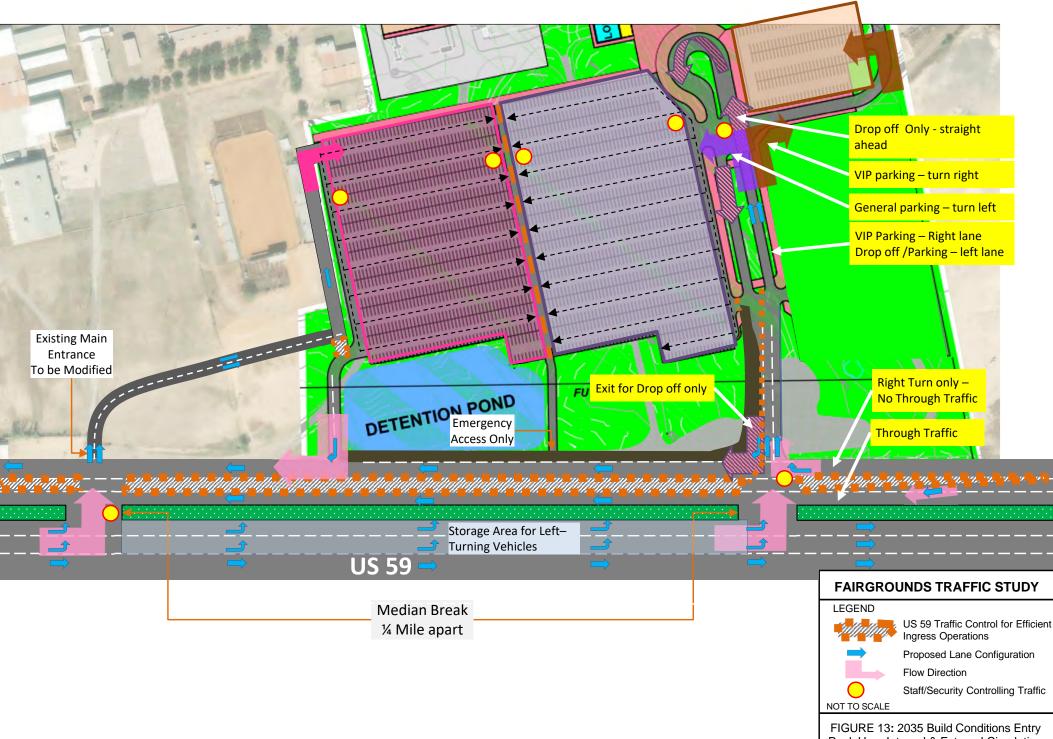
Entry Peak Hour internal/external circulation is shown on **Figure 13** under 2035 Build Conditions. Internal circulation and travel movements during this peak hour at each of the entrances is elaborated below.

Proposed Main Entrance and US 59: Traffic at this intersection will be controlled by staff/security to direct Left-Turns from US 59 northbound and the through traffic on US 59 SB. US 59 SB traffic entering the Fairgrounds is limited to the Proposed Main Entrance. Out of four lanes, two-lanes are dedicated to incoming traffic at the entrance and one-lane is dedicated to the outgoing traffic. Only outgoing traffic at this entrance is drop-offs'/taxi/limo services. Signs will be posted for the incoming traffic limiting VIP parking to the Right-Lane general parking and drop-offs to the Left-Lane. Drop off traffic will be permitted to enter the turn-around at the main entrance and exit the Fairgrounds by making a Right-Turn. Vehicles entering the general parking turn left at the northern most entrance. General parking lot will be managed by staff/security such that the parking spots are utilized effectively and in a synchronized manner.

Proposed Entrance 2: Entrance 2 is restricted to emergency access only.

Proposed Entrance 3: As mentioned in section 6.2, 455 vehicles will be exiting the Fairgrounds during the Entry Peak Hour and all the exit movements will be restricted to utilize Proposed Entrance 3. Entrance 3 Driveway is two-lanes but will be restricted to one-lane during Entry Peak Hour operations.

Existing Main Entrance and US 59: Existing driveway at this entrance will be reconfigured to meet the Entrance 3 driveway. Traffic movement at this intersection will be controlled by staff/security to direct Left-Turns from US 59 northbound and the through traffic on US 59 SB. This entrance is limited to general parking of ingress operations only. This entrance will have two-lanes and both lanes will be dedicated to incoming traffic during Entry Peak Hour. General parking lot will again be managed by staff/security so the parking spots are utilized effectively and in synchronized manner.



Peak Hour Internal & External Circulation



9.2 Exit Peak Hour Operations

Exit Peak Hour internal/external circulation is shown on **Figure 14** under 2035 Build Conditions. Internal circulation and travel movements during this peak hour at each of the entrances is elaborated below.

Proposed Main Entrance and US 59: Traffic at this intersection will be controlled by staff/security as it is during Entry Peak Hour operations to direct Left-Turns from US 59 northbound pick-up/taxi/limo service and the through traffic on US 59 SB. Out of four lanes, two-lanes are dedicated to outgoing Right-Turn movement at the entrance, one-lane is dedicated to outgoing Left-Turn movement, and one-lane for incoming pick-up/taxi/limo service. Only incoming traffic at this entrance is drop-offs'/ taxi/ limo services. Vehicles exiting the general parking can use one of the two exits depending on their parked location. The parking lot will be managed by staff/security for safe and effective flow of the traffic.

Proposed Entrance 2: Entrance 2 is restricted to emergency access only.

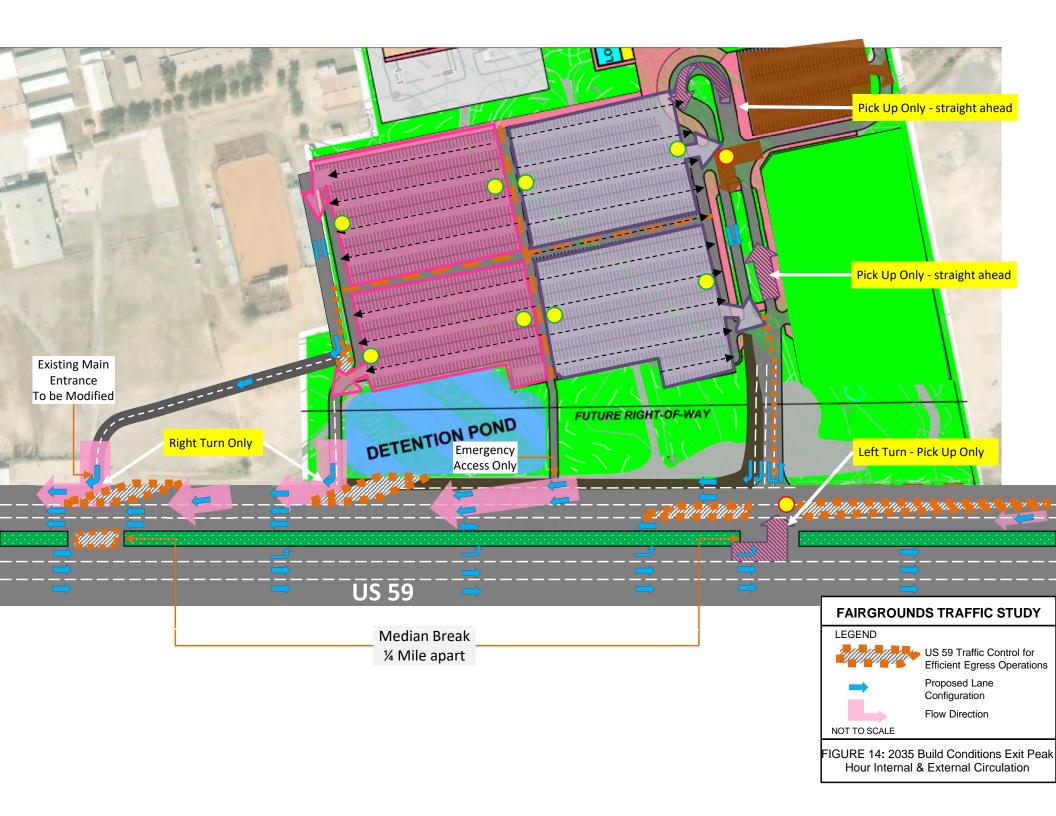
Proposed Entrance 3: Entrance 3 Driveway is two-lane but only one-lane will be utilized during Exit Peak Hour operations. All traffic exiting will be limited to Right-Turns.

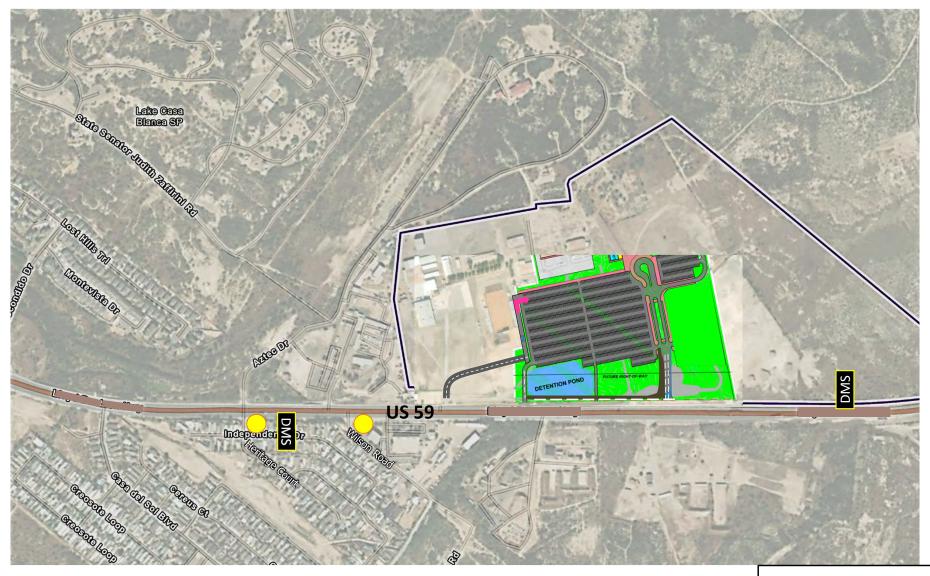
Existing Main Entrance and US 59: This two-lane entrance will be utilizing only one-lane during Exit Peak Hour operations and will be limited to Right-Turns.

9.3 Operations Along US 59/IH 69

Operations along US 59/IH 69 will be impacted during the events based on the proposed lane configuration under 2025 and 2035 Build Conditions. The scope of this study does not include Traffic Management along US 59/IH 69 and it is recommended that a detailed Traffic Management Plan be developed for events at the Fairgrounds.

This traffic study recognizes the need for dynamic message signs (DMSs) along US 59 NB and US 59 SB to alert/warn oncoming traffic of lane closures and altered traffic controls due to a major event. **Figure 15** Traffic Management Sign Locations suggests approximate location of DMS signs along US 59 and event warning signs along Heritage Court and Wilson Road.





Note:

- DMS sign along US 59 NB and US 59 SB will operate during major events when lane closures and altered traffic controls are necessary.
- Placement of DMS sign location is representation purpose only. Traffic Management Plan in the future will define accurate placement and message to be displayed.
- Event warning signs need to be part of Traffic Management Plan.

FAIRGROUNDS TRAFFIC STUDY

LEGEND



Suggested DMS Signs location – to managed by TxDOT

Event Warning signs

NOT TO SCALE

FIGURE 15: 2025 & 2035 Build Conditions Traffic Management Sign Locations



10. Recommendations and Conclusions

10.1 Recommended Improvements

After evaluating 2025 Build Conditions and 2035 Build Conditions with TxDOT improvements, the following recommendations are proposed.

- Proposed Main Entrance Driveway is assumed to be 4 lanes, 2 lanes in each direction with option to do lane revisions depending on the major traffic flow direction.
 - General Engineer Contractor should request Webb County consultant to ensure that driveway is wide enough to accommodate number of lanes and turning radius to accommodate 18-wheeler trucks and school buses.
- Entrance 2 Emergency entrance/exit only.
- Entrance 3 Extending driveway close to the Arena to increase on site storage and reduce queuing on US 59.
- Exiting Entrance 2 Enhance entrance and driveway to meet Entrance 3 driveway as shown in Figures 10 and 11.
- Add an auxiliary lane between proposed main entrance and existing main entrance as indicated on Figures 10 and 11.

10.2 Conclusions

Lane configurations for both Entry and Exit Peak Hours that satisfy the five key factors below will help accomplish safe and efficient operations for all entrances and US 59.

- Utilize Proposed Main Entrance and existing Main Entrance for ingress and Proposed Entrance 3 for egress during Entry Peak Hour to **minimize conflicting movements**.
- Designate Proposed Main Entrance, Proposed Entrance 3, and existing Main Entrance for exit trips during Exit Peak Hour to allow sufficient distance between the exits for traffic to merge on to US 59 while maximizing safety along US 59.
- Propose Main Entrance 2,500 feet away from Wilson Road and designate two Left-Turn lanes during Entry Peak Hour to double the storage capacity and allow vehicles to enter the Fairgrounds quickly in order to alleviate sight distance challenges for Wilson Road and Heritage Court.
- Provide two long driveways on either side of the parking lot to facilitate **efficient internal circulation** and provide storage when necessary, during Entry and Exit Peak Hours.
- During Entry and Exit Peak Hours, operate Proposed Entrance 2 as **emergency access** to disperse traffic efficiently and as exclusive access to EMS vehicles during emergencies.

In addition, utilize the following traffic control measures for safe/quick ingress/egress to the Fairgrounds:

- Staff/security control of inbound and outbound traffic from/to US 59
- Lane revisions/enhancements using traffic cones and flares to temporarily "add/remove" lanes
- Staff/security directing to/from parking lots
- Signs to direct traffic to/from parking areas
- DMSs along US 59 to alert/warn oncoming traffic of lane closures and altered traffic controls
- Event warning signs on cross streets.

Finally, based on the traffic analysis conducted to determine the impacts on study area by the proposed Site Plan, the public roadway system serving the site as well as the study intersections can accommodate traffic generated at the Fairgrounds during major events provided the above recommended improvements in Section 10.1 are considered for implementation.

Appendix A: Traffic Count Data



Fri Mar 4, 2022

Full Length (12 AM-12 AM (+1))

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927846, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	T	Арр	T	Арр	Int
2022-03-04 12:00AN	M 29	29	18	18	47
1:00AN	M 19	19	19	19	38
2:00AN	M 12	12	12	12	24
3:00AN	M 15	15	5	5	20
4:00Al	M 15	15	34	34	49
5:00AN	M 38	38	50	50	88
6:00AN	M 63	63	80	80	143
7:00AN	M 124	124	87	87	211
8:00Al	M 137	137	144	144	281
9:00AN	M 142	142	109	109	251
10:00AN	M 146	146	119	119	265
11:00AN	M 164	164	132	132	296
12:00P!	M 127	127	146	146	273
1:00P!	M 134	134	149	149	283
2:00P!	M 169	169	133	133	302
3:00P!	M 164	164	165	165	329
4:00P!	M 198	198	175	175	373
5:00P1	M 169	169	153	153	322
6:00P!	M 195	195	135	135	330
7:00PI	M 145	145	125	125	270
8:00P1	M 104	104	78	78	182
9:00P1	M 94	94	54	54	148
10:00P	M 55	55	29	29	84
11:00P	M 34	34	32	32	66
Tota	al 2492	2492	2183	2183	4675
% Approac	h 100%	-	100%	-	-
% Tota	al 53.3%	53.3%	46.7%	46.7%	-
Light	ts 1937	1937	1786	1786	3723
% Light	rs 77.7%	77.7%	81.8%	81.8%	79.6%
Articulated Truck	s 461	461	303	303	764
% Articulated Truck	s 18.5%	18.5%	13.9%	13.9%	16.3%
Buses and Single-Unit Truck	s 94	94	94	94	188
% Buses and Single-Unit Truck	s 3.8%	3.8%	4.3%	4.3%	4.0%

^{*}T: Thru

Fri Mar 4, 2022 Full Length (12 AM-12 AM (+1))

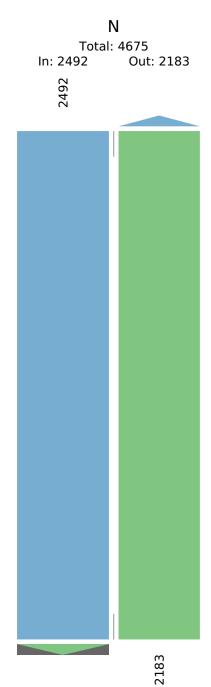
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927846, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US



Out: 2492 In: 2183 Total: 4675 S

Fri Mar 4, 2022

AM Peak (Mar 04 2022 8:15AM - 9:15 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927846, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	Т	Арр	T	Арр	Int
2022-03-04 8:15AM	37	37	41	41	78
8:30AM	36	36	33	33	69
8:45AM	34	34	37	37	71
9:00AM	38	38	33	33	71
Total	145	145	144	144	289
% Approach	100%	-	100%	-	-
% Total	50.2%	50.2%	49.8%	49.8%	-
PHI	0.954	0.954	0.878	0.878	0.926
Lights	114	114	118	118	232
% Lights	78.6%	78.6%	81.9%	81.9%	80.3%
Articulated Trucks	25	25	21	21	46
% Articulated Trucks	17.2%	17.2%	14.6%	14.6%	15.9%
Buses and Single-Unit Trucks	6	6	5	5	11
% Buses and Single-Unit Trucks	4.1%	4.1%	3.5%	3.5%	3.8%

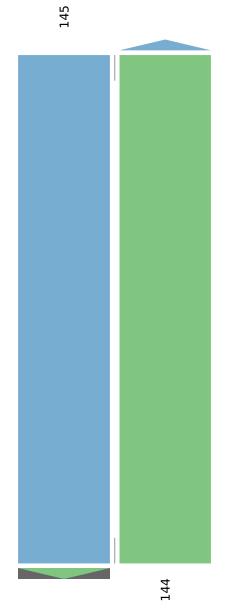
^{*}T: Thru

Fri Mar 4, 2022 AM Peak (Mar 04 2022 8:15AM - 9:15 AM) All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks) All Channels ID: 927846, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 289 In: 145 Out: 144



Out: 145 In: 144 Total: 289 S

Fri Mar 4, 2022

Midday Peak (Mar 04 2022 11AM - 12 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927846, Location: 27.5379, -99.419616



Leg		North		South		
Direction		Southbound		Northbound		
Time		T	Арр	T	Арр	Int
	2022-03-04 11:00AM	50	50	29	29	79
	11:15AM	43	43	37	37	80
	11:30AM	31	31	39	39	70
	11:45AM	40	40	27	27	67
	Total	164	164	132	132	296
	% Approach	100%	-	100%	-	-
	% Total	55.4%	55.4%	44.6%	44.6%	-
	PHF	0.820	0.820	0.846	0.846	0.925
	Lights	128	128	95	95	223
	% Lights	78.0%	78.0%	72.0%	72.0%	75.3%
	Articulated Trucks	28	28	26	26	54
	% Articulated Trucks	17.1%	17.1%	19.7%	19.7%	18.2%
	Buses and Single-Unit Trucks	8	8	11	11	19
%	Buses and Single-Unit Trucks	4.9%	4.9%	8.3%	8.3%	6.4%

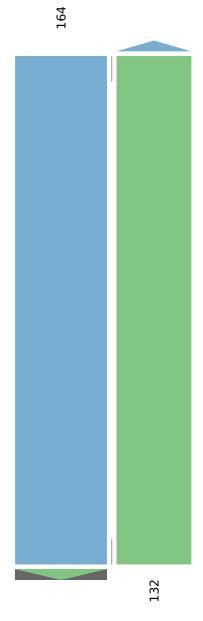
^{*}T: Thru

Fri Mar 4, 2022 Midday Peak (Mar 04 2022 11AM - 12 PM) All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks) All Channels ID: 927846, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 296 In: 164 Out: 132



Out: 164 In: 132 Total: 296 S

Fri Mar 4, 2022

PM Peak (Mar 04 2022 4PM - 5 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927846, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	T	Арр	T	Арр	Int
2022-03-04 4:00P	<i>A</i> 58	58	50	50	108
4:15P	<i>A</i> 30	30	43	43	73
4:30Pl	Л 48	48	30	30	78
4:45P	A 62	62	52	52	114
Tot	1 98	198	175	175	373
% Approac	h 100%	-	100%	-	-
% Tot	i 53.1%	53.1%	46.9%	46.9%	-
PH	F 0.798	0.798	0.841	0.841	0.818
Ligh	s 156	156	149	149	305
% Ligh	s 78.8%	78.8%	85.1%	85.1%	81.8%
Articulated Truck	s 36	36	20	20	56
% Articulated Truck	s 18.2%	18.2%	11.4%	11.4%	15.0%
Buses and Single-Unit Truck	s 6	6	6	6	12
% Buses and Single-Unit Truck	3.0%	3.0%	3.4%	3.4%	3.2%

^{*}T: Thru

Fri Mar 4, 2022

PM Peak (Mar 04 2022 4PM - 5 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

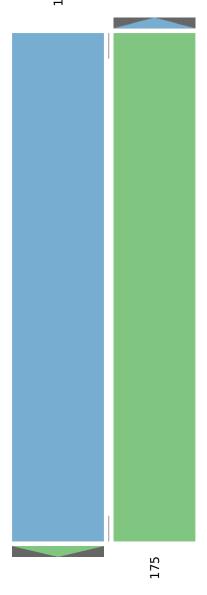
ID: 927846, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 373 In: 198 Out: 175

198



Out: 198 In: 175 Total: 373 S

Sat Mar 5, 2022

Full Length (12 AM-12 AM (+1))

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927847, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	T	Арр	T	Арр	Int
2022-03-05 12:00AM	18	18	36	36	54
1:00AM	19	19	23	23	42
2:00AM	19	19	8	8	27
3:00AM	13	13	13	13	26
4:00AM	8	8	13	13	21
5:00AM	23	23	39	39	62
6:00AM	30	30	64	64	94
7:00AM	51	51	102	102	153
8:00AM	96	96	133	133	229
9:00AM	114	114	173	173	287
10:00AM	115	115	146	146	261
11:00AM	159	159	155	155	314
12:00PM	125	125	177	177	302
1:00PM	132	132	171	171	303
2:00PM	122	122	175	175	297
3:00PM	110	110	159	159	269
4:00PM	149	149	146	146	295
5:00PM	157	157	135	135	292
6:00PM	143	143	108	108	251
7:00PM	144	144	60	60	204
8:00PM	147	147	73	73	220
9:00PM	151	151	56	56	207
10:00PM	96	96	42	42	138
11:00PM	72	72	32	32	104
Total	2213	2213	2239	2239	4452
% Approach	100%	-	100%	-	-
% Total	49.7%	49.7%	50.3%	50.3%	-
Lights	1997	1997	2058	2058	4055
% Lights	90.2%	90.2%	91.9%	91.9%	91.1%
Articulated Trucks	174	174	133	133	307
% Articulated Trucks	7.9%	7.9%	5.9%	5.9%	6.9%
Buses and Single-Unit Trucks	42	42	48	48	90
% Buses and Single-Unit Trucks	1.9%	1.9%	2.1%	2.1%	2.0%

^{*}T: Thru

Sat Mar 5, 2022 Full Length (12 AM-12 AM (+1))

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

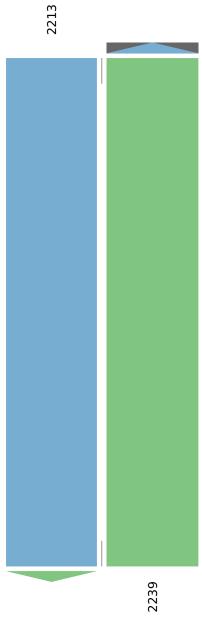
All Channels

ID: 927847, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 4452 In: 2213 Out: 2239



Out: 2213 In: 2239 Total: 4452 S

Sat Mar 5, 2022

AM Peak (WKND) (9 AM - 10 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927847, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	T	Арр	T	Арр	Int
2022-03-05 9:00AM	25	25	36	36	61
9:15AM	26	26	38	38	64
9:30AM	33	33	48	48	81
9:45AM	30	30	51	51	81
Total	114	114	173	173	287
% Approach	100%	-	100%	-	-
% Total	39.7%	39.7%	60.3%	60.3%	-
PHI	0.864	0.864	0.848	0.848	0.886
Lights	94	94	167	167	261
% Lights	82.5%	82.5%	96.5%	96.5%	90.9%
Articulated Trucks	18	18	5	5	23
% Articulated Trucks	15.8%	15.8%	2.9%	2.9%	8.0%
Buses and Single-Unit Trucks	2	2	1	1	3
% Buses and Single-Unit Trucks	1.8%	1.8%	0.6%	0.6%	1.0%

^{*}T: Thru

Sat Mar 5, 2022 AM Peak (WKND) (9 AM - 10 AM) All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks) All Channels ID: 927847, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 287 In: 114 Out: 173



Out: 114 In: 173 Total: 287

Sat Mar 5, 2022

Midday Peak (WKND) (12:45 PM - 1:45 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927847, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	T	Арр	T	Арр	Int
2022-03-05 12:45P	43	43	52	52	95
1:00PI	4 30	30	43	43	73
1:15P	1 24	24	50	50	74
1:30PI	46	46	43	43	89
Total	l 143	143	188	188	331
% Approac	h 100%	-	100%	-	-
% Total	d 43.2%	43.2%	56.8%	56.8%	-
PH	F 0.777	0.777	0.904	0.904	0.871
Light	s 131	131	175	175	306
% Light	s 91.6%	91.6%	93.1%	93.1%	92.4%
Articulated Truck	s 11	11	10	10	21
% Articulated Truck	7.7%	7.7%	5.3%	5.3%	6.3%
Buses and Single-Unit Truck	1	1	3	3	4
% Buses and Single-Unit Truck	0.7%	0.7%	1.6%	1.6%	1.2%

^{*}T: Thru

Sat Mar 5, 2022 Midday Peak (WKND) (12:45 PM - 1:45 PM) - Overall Peak Hour All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks) All Channels

ID: 927847, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 331 In: 143 Out: 188

143



Out: 143 In: 188 Total: 331

Sat Mar 5, 2022

PM Peak (WKND) (3:45 PM - 4:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

ID: 927847, Location: 27.5379, -99.419616



Leg	North		South		
Direction	Southbound		Northbound		
Time	Т	Арр	T	Арр	Int
2022-03-05 3:45PM	1 38	38	47	47	85
4:00PM	1 44	44	25	25	69
4:15PM	46	46	42	42	88
4:30PM	1 23	23	48	48	71
Tota	l 151	151	162	162	313
% Approac	h 100%	-	100%	-	-
% Tota	l 48.2%	48.2%	51.8%	51.8%	-
PH	F 0.821	0.821	0.844	0.844	0.889
Light	s 143	143	150	150	293
% Light	94.7%	94.7%	92.6%	92.6%	93.6%
Articulated Truck	s 7	7	5	5	12
% Articulated Truck	4.6%	4.6%	3.1%	3.1%	3.8%
Buses and Single-Unit Truck	1	1	7	7	8
% Buses and Single-Unit Truck	0.7%	0.7%	4.3%	4.3%	2.6%

^{*}T: Thru

Sat Mar 5, 2022 PM Peak (WKND) (3:45 PM - 4:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks)

All Channels

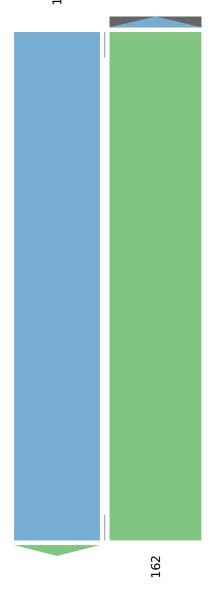
ID: 927847, Location: 27.5379, -99.419616



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

N Total: 313 In: 151 Out: 162

151



Out: 151 In: 162 Total: 313 S

US 59 at Entrance 1 - add'l - TMC

Sat Mar 5, 2022 Full Length (12 AM-1 AM)

Full Length (12 AM-1 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 975541, Location: 27.536488, -99.42162



Leg	US 59)				US 59					Entrance 1					
Direction	South	bound				Northbour	nd				Eastbound					
Time	R	T	U	App	Ped*	Т	L	U	Арр	Ped*	R	L	U	App	Ped*	Int
2022-03-05 12:00AM	0	18	0	18	0	35	4	0	39	0	15	1	0	16	0	73
Total	0	18	0	18	0	35	4	0	39	0	15	1	0	16	0	73
% Approach	0%	100%	0%	-	-	89.7%	10.3%	0%	-	-	93.8%	6.3%	0%	-	-	-
% Total	0%	24.7%	0%	24.7%	-	47.9%	5.5%	0%	53.4%	-	20.5%	1.4%	0%	21.9%	-	-
Lights	0	13	0	13	-	31	4	0	35	-	15	1	0	16	-	64
% Lights	0%	72.2%	0%	72.2%	-	88.6%	100%	0%	89.7%	-	100%	100%	0%	100%	-	87.7%
Articulated Trucks	0	4	0	4	-	4	0	0	4	-	0	0	0	0	-	8
% Articulated Trucks	0%	22.2%	0%	22.2%	-	11.4%	0%	0%	10.3%	-	0%	0%	0%	0%	-	11.0%
Buses and Single-Unit Trucks	0	1	0	1	-	0	0	0	0	-	0	0	0	0	-	1
% Buses and Single-Unit Trucks	0%	5.6%	0%	5.6%	-	0%	0%	0%	0%	-	0%	0%	0%	0%	-	1.4%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

US 59 at Entrance 1 - add'l - TMC

Sat Mar 5, 2022

Full Length (12 AM-1 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

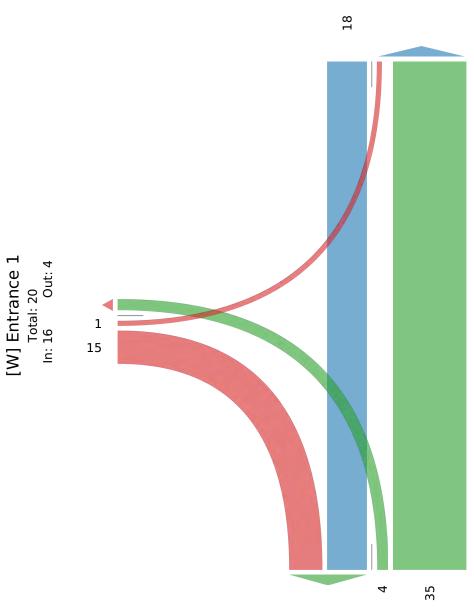
All Movements

ID: 975541, Location: 27.536488, -99.42162



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US





Out: 33 In: 39 Total: 72 [S] US 59

US 59 at Entrance 1 - TMC

Sat Mar 5, 2022

Full Length (7:45 PM-8:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians,

Bicycles on Crosswalk) All Movements

ID: 927843, Location: 27.536488, -99.42162



Leg	US 59					US 59					Entrance	1				
Direction	Southbou	ınd				Northbour	nd				Eastbour	nd				
Time	R	T	U	App	Ped*	T	L	U	App	Ped*	R	L	U	Арр	Ped*	Int
2022-03-05 7:45PM	0	48	0	48	0	14	2	0	16	0	1	0	0	1	0	65
8:00PM	0	38	0	38	0	17	2	0	19	0	0	0	0	0	0	57
8:15PM	1	46	0	47	0	17	4	2	23	0	2	0	0	2	0	72
8:30PM	0	30	0	30	0	26	1	2	29	0	2	0	0	2	0	61
Total	1	162	0	163	0	74	9	4	87	0	5	0	0	5	0	255
% Approach	0.6%	99.4%	0%	-	-	85.1%	10.3%	4.6%	-	-	100%	0%	0%	-	-	-
% Total	0.4%	63.5%	0%	63.9%	-	29.0%	3.5%	1.6%	34.1%	-	2.0%	0%	0%	2.0%	-	-
PHF	0.250	0.844	-	0.849	-	0.712	0.563	0.500	0.750	-	0.625	-	-	0.625	-	0.885
Lights	1	157	0	158	-	70	9	4	83	-	5	0	0	5	-	246
% Lights	100%	96.9%	0%	96.9%	-	94.6%	100%	100%	95.4%	-	100%	0%	0%	100%	-	96.5%
Articulated Trucks	0	2	0	2	-	1	0	0	1	-	0	0	0	0	-	3
% Articulated Trucks	0%	1.2%	0%	1.2%	-	1.4%	0%	0%	1.1%	-	0%	0%	0%	0%	-	1.2%
Buses and Single-Unit Trucks	0	3	0	3	-	3	0	0	3	-	0	0	0	0	-	6
% Buses and Single-Unit Trucks	0%	1.9%	0%	1.8%	-	4.1%	0%	0%	3.4%	-	0%	0%	0%	0%	-	2.4%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

US 59 at Entrance 1 - TMC

Sat Mar 5, 2022

Full Length (7:45 PM-8:45 PM)

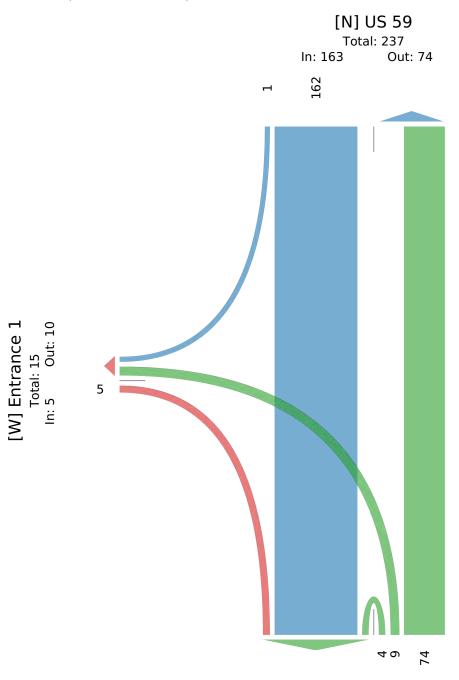
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927843, Location: 27.536488, -99.42162



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US



Out: 171 In: 87 Total: 258 [S] US 59

US 59 at Entrance 2 - add'l - TMC

Sat Mar 5, 2022 Full Length (12 AM-1 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians,

Bicycles on Crosswalk)

All Movements

ID: 975543, Location: 27.534036, -99.424882



Provided by: C. J. Hensch & Associates 5215 Sycamore Ave.,

ID: 975543, Location: 27.534036	5, -99.4	24882											Pasac	lena, Ťž	K, 775	03, US
Leg	US 59					US 59					Entrance :	2				
Direction	Southbo	und				Northbou	nd				Eastbound	d				
Time	R	T	U	App	Ped*	T	L	U	App	Ped*	R	L	U	App	Ped*	Int
2022-03-05 12:00AM	1	32	0	33	0	30	21	0	51	0	527	6	1	534	3	618
Total	1	32	0	33	0	30	21	0	51	0	527	6	1	534	3	618
% Approach	3.0%	97.0%	0%	-	-	58.8%	41.2%	0%	-	-	98.7%	1.1%	0.2%	-	-	-
% Total	0.2%	5.2%	0%	5.3%	-	4.9%	3.4%	0%	8.3%	-	85.3%	1.0%	0.2%	86.4%	-	-
Lights	1	28	0	29	-	28	21	0	49	-	527	6	1	534	-	612
% Lights	100%	87.5%	0%	87.9%	-	93.3%	100%	0%	96.1%	-	100%	100%	100%	100%	-	99.0%
Articulated Trucks	0	4	0	4	-	2	0	0	2	-	0	0	0	0	-	6
% Articulated Trucks	0%	12.5%	0%	12.1%	-	6.7%	0%	0%	3.9%	-	0%	0%	0%	0%	-	1.0%
Buses and Single-Unit Trucks	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Buses and Single-Unit Trucks	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	3	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100%	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

US 59 at Entrance 2 - add'l - TMC

Sat Mar 5, 2022

Full Length (12 AM-1 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

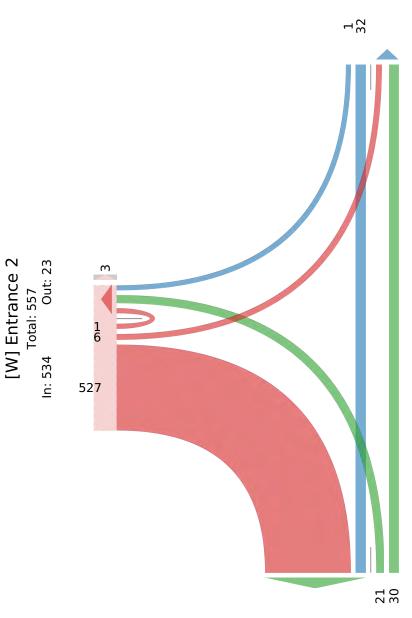
All Movements

ID: 975543, Location: 27.534036, -99.424882



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

[N] US 59 Total: 69 In: 33 Out: 36



Out: 559 In: 51 Total: 610 [S] US 59

US 59 at Entrance 2 - TMC

Sat Mar 5, 2022

Full Length (7:45 PM-8:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927842, Location: 27.534036, -99.424882



Leg	US 59					US 59					Entrance	2				
Direction	Southbou	ınd				Northbou	nd				Eastboun	d				ı
Time	R	T	U	App	Ped*	T	L	U	App	Ped*	R	L	U	App	Ped*	Int
2022-03-05 7:00PM	0	45	0	45	0	17	127	0	144	0	25	0	0	25	0	214
8:00PM	12	118	0	130	0	77	412	0	489	0	107	0	1	108	0	727
Total	12	163	0	175	0	94	539	0	633	0	132	0	1	133	0	941
% Approach	6.9%	93.1%	0%	-	-	14.8%	85.2%	0%	-	-	99.2%	0%	0.8%	-	-	-
% Total	1.3%	17.3%	0%	18.6%	-	10.0%	57.3%	0%	67.3%	-	14.0%	0%	0.1%	14.1%	-	-
Lights	12	160	0	172	-	88	538	0	626	-	132	0	0	132	-	930
% Lights	100%	98.2%	0%	98.3%	-	93.6%	99.8%	0%	98.9%	-	100%	0%	0%	99.2%	-	98.8%
Articulated Trucks	0	1	0	1	-	2	0	0	2	-	0	0	0	0	-	3
% Articulated Trucks	0%	0.6%	0%	0.6%	-	2.1%	0%	0%	0.3%	-	0%	0%	0%	0%	-	0.3%
Buses and Single-Unit Trucks	0	2	0	2	-	4	1	0	5	-	0	0	1	1	-	8
% Buses and Single-Unit Trucks	0%	1.2%	0%	1.1%	-	4.3%	0.2%	0%	0.8%	-	0%	0%	100%	0.8%	-	0.9%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-		-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

US 59 at Entrance 2 - TMC

Sat Mar 5, 2022

Full Length (7:45 PM-8:45 PM)

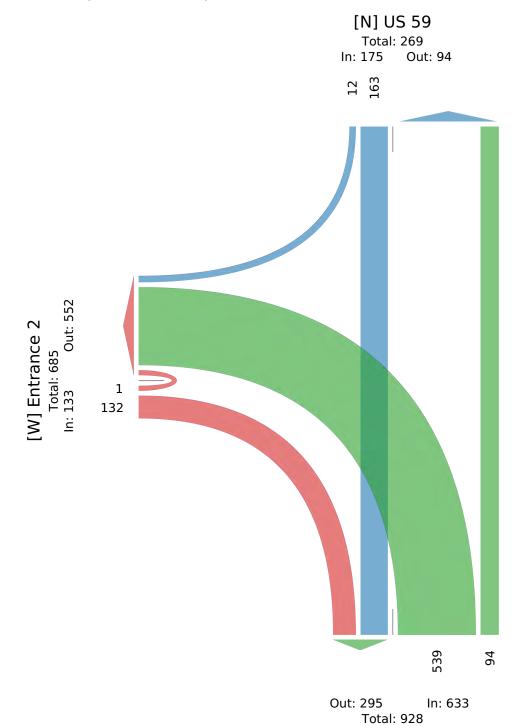
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927842, Location: 27.534036, -99.424882



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US



[S] US 59

2 of 4

US 59 at Entrance 3 - add'l - TMC

Sat Mar 5, 2022 Full Length (12 AM-1 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians,

Bicycles on Crosswalk)

All Movements

ID: 975546, Location: 27.532723, -99.426673



Pasadena, TX, 77503, US

Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave.,

-								- 1											_						
Leg	US 5	59					Wilson	Rd					US 59						Entrar	ice :	3				
Direction	Sout	hbour	ıd				Westbo	und					Northb	ound					Eastbo	ounc	l				
Time	R	Т	L	U	App	Ped*	R	T	L	U	App P	ed*	R	T	L	U	App P	ed*	R	T	L	U	App 1	Ped*	Int
2022-03-05 12:00AM	0	548	0	0	548	0	1	0	1	0	2	0	1	50	0	0	51	0	1	0	0	0	1	0	602
Total	0	548	0	0	548	0	1	0	1	0	2	0	1	50	0	0	51	0	1	0	0	0	1	0	602
% Approach	0%	100%	0%	0%	-	-	50.0%	0%	50.0%)%	-	-	2.0% 9	98.0% (0% ()%	-	-	100%	0%	0%	0%	-	-	-
% Total	0% 9	91.0%	0%	0%	91.0%	-	0.2%	0%	0.2%)%	0.3%	-	0.2%	8.3%	0% ()%	8.5%	-	0.2%	0%	0%	0%	0.2%	-	-
Lights	0	539	0	0	539	-	1	0	1	0	2	-	1	48	0	0	49	-	1	0	0	0	1	-	591
% Lights	0% 9	98.4%	0%	0%	98.4%	-	100%	0%	100%)%	100%	-	100% 9	96.0% (0% ()% 9	96.1%	-	100%	0%	0%	0%	100%	-	98.2%
Articulated Trucks	0	5	0	0	5	-	0	0	0	0	0	-	0	2	0	0	2	-	0	0	0	0	0	-	7
% Articulated Trucks	0%	0.9%	0%	0%	0.9%	-	0%	0%	0% ()%	0%	-	0%	4.0%	0% ()%	3.9%	-	0%	0%	0%	0%	0%	-	1.2%
Buses and Single-Unit Trucks	0	4	0	0	4	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	4
% Buses and Single-Unit Trucks	0%	0.7%	0%	0%	0.7%	-	0%	0%	0% ()%	0%	-	0%	0% (0% ()%	0%	-	0%	0%	0%	0%	0%	-	0.7%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-		-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

US 59 at Entrance 3 - add'l - TMC

Sat Mar 5, 2022

Full Length (12 AM-1 AM)

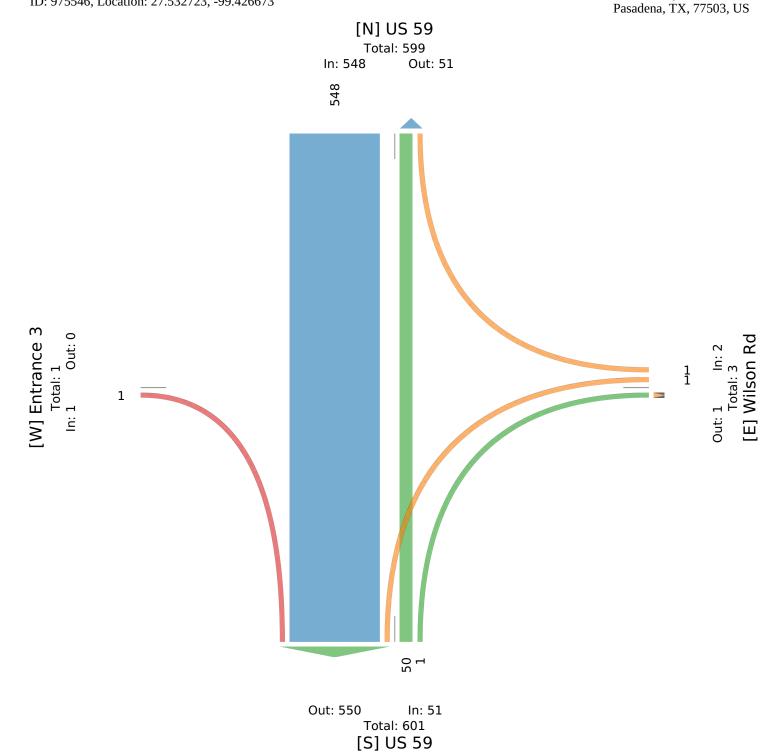
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 975546, Location: 27.532723, -99.426673



Provided by: C. J. Hensch & Associates 5215 Sycamore Ave.,



US 59 at Entrance 3 - TMC

Sat Mar 5, 2022

Full Length (7:45 PM-8:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927841, Location: 27.532723, -99.426673



Leg	US 59						Wilson	Rd					US 59						Entrand	e 3					
Direction	Southb	ound					Westbo	und					Northbo	ound					Eastbo	ınd					
Time	R	T	L	U	App P	ed*	R	T	L	U	App	Ped*	R	T	L	U	App	Ped*	R	T	L	U	App P	ed*	Int
2022-03-05 7:00PM	1	68	0	0	69	0	1	0	1	0	2	0	1	144	2	0	147	0	1	0	0	0	1	0	219
8:00PM	0	219	0	0	219	0	2	0	2	0	4	0	4	487	0	0	491	0	2	0	0	0	2	0	716
Total	1	287	0	0	288	0	3	0	3	0	6	0	5	631	2	0	638	0	3	0	0	0	3	0	935
% Approach	0.3%	99.7%	0% (0%	-	-	50.0%	0%	50.0% ()%	-	-	0.8%	98.9%	0.3%	0%	-	-	100%	0%	0% (0%	-	-	-
% Total	0.1%	30.7%	0% (0% 3	80.8%	-	0.3%	0%	0.3% ()%	0.6%	-	0.5%	67.5%	0.2%	0% (68.2%	-	0.3%	0%	0% (0%	0.3%	-	-
Lights	1	283	0	0	284	-	3	0	3	0	6	-	4	626	0	0	630	-	1	0	0	0	1	-	921
% Lights	100% 9	98.6%	0%	0% 9	98.6%	-	100%	0%	100% ()% :	100%	-	80.0%	99.2%	0%	0% 9	98.7%	-	33.3%	0%	0% (0% 3	33.3%	-	98.5%
Articulated Trucks	0	2	0	0	2	-	0	0	0	0	0	-	0	2	0	0	2	-	0	0	0	0	0	-	4
% Articulated Trucks	0%	0.7%	0% (0%	0.7%	-	0%	0%	0% ()%	0%	-	0%	0.3%	0%	0%	0.3%	-	0%	0%	0% (0%	0%	-	0.4%
Buses and Single-Unit Trucks	0	2	0	0	2	-	0	0	0	0	0	-	1	3	2	0	6	-	2	0	0	0	2	-	10
% Buses and Single-Unit Trucks		0.7%	0% (0%	0.7%	-	0%	0%	0% ()%	0%	-	20.0%	0.5%	100%	0%	0.9%	-	66.7%	0%	0% (0% (66.7%	-	1.1%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	_	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

US 59 at Entrance 3 - TMC

Sat Mar 5, 2022

Full Length (7:45 PM-8:45 PM)

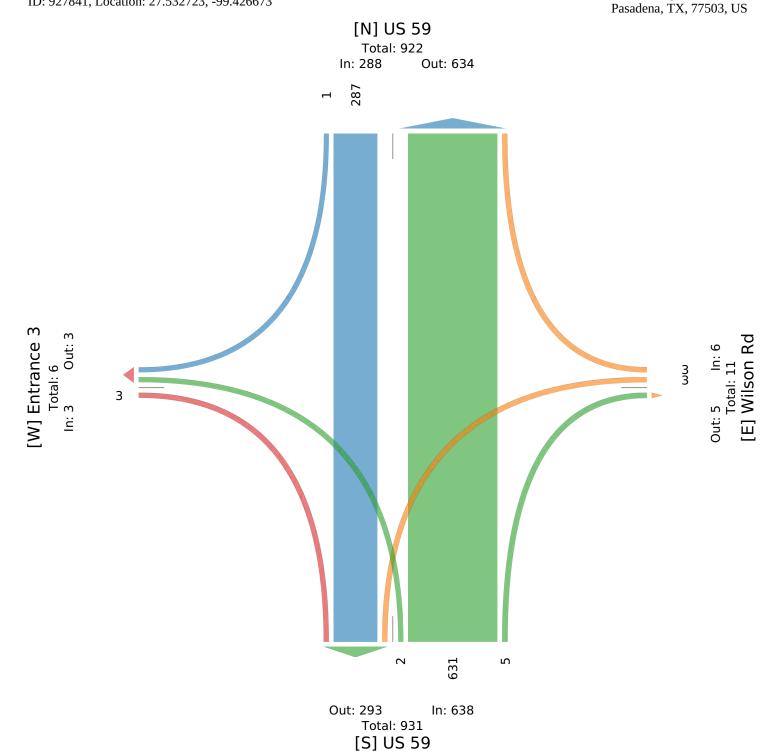
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927841, Location: 27.532723, -99.426673



Provided by: C. J. Hensch & Associates 5215 Sycamore Ave.,



Fri Mar 4, 2022

Full Length (12 AM-12 AM (+1))

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927844, Location: 27.531324, -99.428482



Leg	US 59	,				Cuella	r Roady	tate Rep way	oresent	ative H	enry	US 59	1					Heritage Cuellar	Roadv		resent	ative H	enry	
Direction	Southb				• 5 1	Westb					70 14	Northbo					2.10	Eastbou					D 10	
Time	R	T	L	U	App Ped*	R	T	L	U	App	Ped*	R	T	L	U	App 1		R	T	L	U	App	_	
2022-03-04 12:00AM	0	260	0	0	260 0	_		9	0	9	0	11	23	1	0	35	0	12	0	3	0	15	0	319
1:00AM	0	33	0	0	33 0	_	0	5	0	5	0	8	17	0	0	25	0	1	0	0	0	1	0	64
2:00AM	0	17	0	0	17 0	_		2	0	2	0	2	18	0	0	20	0	0	0	0	0	0	0	39
3:00AM	0	18	0	0	18 0			0	0	0	0	0	6	0	0	6	0	0	0	0	0	0	0	24
4:00AM	0	15	0	0	15 0	_		7	0	7	0	3	35	0	0	38	0	0	0	0	0	0	0	60
5:00AM	0	42	0	0	42 0			17	0	17	0	2	84	1	0	87	0	0	0	0	0	0	0	146
6:00AM	0	87	0	0	87 0	1		26	0	26	0	11	114	1	0	126	0	3	0	0	0	3	0	242
7:00AM	1	150	2	0	153 0		0	78	0	79	0	38	185	9	1	233	0	1	0	0	0	1	0	466
8:00AM	0	235	0	0	235 1	_		51	0	51	1	24	325	10	0	359	0	6	0	2	0	8	0	653
9:00AM	0	222	1	0	223 0	1		35	0	35	0	27	277	11	0	315	0	2	0	1	0	3	0	576
10:00AM	1	248	0	0	249 0	_		36	0	37	0	24	287	7	0	318	0	9	0	2	0	11	0	615
11:00AM	0	268	2	0	270 0			34	0	34	0	32	279	7	0	318	0	8	0	0	0	8	0	630
12:00PM	1	281	0	0	282 0	1	0	34	0	35	0	40	345	7	0	392	0	9	0	0	0	9	1	718
1:00PM	1	329	0	0	330 0	1	0	30	0	31	0	23	355	10	0	388	0	3	0	1	0	4	1	753
2:00PM	3	330	1	0	334 0	0	0	31	0	31	0	36	278	5	0	319	0	6	0	0	0	6	0	690
3:00PM	4	326	1	0	331 0	1	0	31	0	32	0	36	311	9	0	356	0	10	0	2	0	12	0	731
4:00PM	0	353	1	0	354 0	1	0	41	0	42	2	43	316	4	0	363	0	4	0	0	0	4	0	763
5:00PM	3	265	2	0	270 0	5	0	42	0	47	2	69	367	2	0	438	0	17	0	0	0	17	0	772
6:00PM	9	308	1	0	318 0	0	0	52	1	53	2	73	454	4	1	532	0	3	0	1	0	4	1	907
7:00PM	0	240	1	0	241 0	7	1	46	0	54	1	63	599	6	0	668	0	3	1	1	0	5	0	968
8:00PM	0	224	1	0	225 0	1	0	41	0	42	0	71	477	11	1	560	0	14	0	1	1	16	0	843
9:00PM	3	266	3	1	273 0	3	0	38	0	41	0	32	266	0	1	299	0	2	0	0	0	2	0	615
10:00PM	2	341	0	0	343 0	0	0	29	0	29	1	30	129	6	0	165	0	3	0	2	0	5	0	542
11:00PM	3	361	0	0	364 0	0	0	12	0	12	0	27	90	4	0	121	0	6	0	0	0	6	0	503
Total	31	5219	16	1	5267 1	. 22	1	727	1	751	9	725	5637	115	4	6481	0	122	1	16	1	140	3	12639
% Approach	0.6%	99.1%	0.3%	0%		2.9%	0.1%	96.8%	0.1%	-	-	11.2%	87.0%	1.8%	0.1%	-	-	87.1%	0.7%	11.4%	0.7%	-	-	-
% Total	0.2%	41.3%	0.1%	0%	41.7%	0.2%	0%	5.8%	0%	5.9%	-	5.7%	44.6%	0.9%	0% 5	51.3%	-	1.0%	0%	0.1%	0%	1.1%	-	-
Lights	29	4627	15	1	4672	- 21	1	696	1	719	-	692	5181	113	3	5989	-	119	1	15	1	136	-	11516
% Lights	93.5%	88.7%	93.8%	100%	88.7%	95.5%	100%	95.7%	100% 9	95.7%	-	95.4%	91.9%	98.3% 7	75.0% 9	92.4%	-	97.5% 1	100% 9	93.8% 1	100% 9	97.1%	-	91.1%
Articulated Trucks	0	451	0	0	451	. 0	0	0	0	0	-	0	314	1	1	316	-	0	0	0	0	0	-	767
% Articulated Trucks	0%	8.6%	0%	0%	8.6%	0%	0%	0%	0%	0%	_	0%	5.6%	0.9% 2	25.0%	4.9%	_	0%	0%	0%	0%	0%	-	6.1%
Buses and Single-Unit																								
Trucks	2	141	1	0	144	1	0	31	0	32	-	33	142	1	0	176	-	3	0	1	0	4	-	356
% Buses and Single-Unit																								
Trucks	6.5%	2.7%	6.3%	0%	2.7%	4.5%	0%	4.3%	0%	4.3%	-	4.6%	2.5%	0.9%	0%	2.7%	-	2.5%	0%	6.3%	0%	2.9%	-	2.8%
Pedestrians	-	-	-	-	- 1	-	-	-	-	-	3	-	-	-	-	-	0	-	-	-	-	-	3	
% Pedestrians	-	-	-	-	- 100%	-	-	-	-	- 3	33.3%	-	-	-	-	-	-	-	-	-	-	- 1	.00%	-
Bicycles on Crosswalk	-	-	-	-	- 0	-	-	-	-	-	6	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	- 0%	-	-	-	-	- (56.7%	-	-	-	-	-	-	-	-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Fri Mar 4, 2022

Full Length (12 AM-12 AM (+1))

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

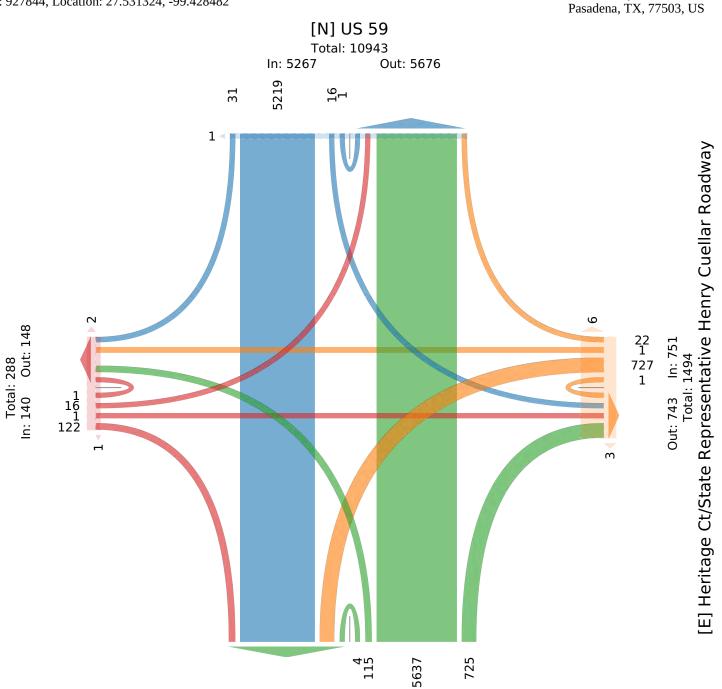
All Movements

[W] Heritage Ct/State Representative Henry Cuellar Roadway

ID: 927844, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave.,



Out: 6072 In: 6481 Total: 12553 [S] US 59

2 of 8

Fri Mar 4, 2022

AM Peak (Mar 04 2022 8:15AM - 9:15 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927844, Location: 27.531324, -99.428482



8	US						Hen	ry Cı	ıellar Ro		resentativ ay	re	US 59						Heritag Repres Roadw	enta ay			ry Cue	llar	
Direction	Sou	thboun	d				Wes	tbou	nd				Northb	ound					Eastbo	und					
Time	R	T	L	U	App 1	Ped*	R	T	L	U	App	Ped*	R	T	L	U	App	Ped*	R	T	L	U	App	Ped*	Int
2022-03-04 8:15AM	0	60	0	0	60	0	0	0	14	0	14	0	7	69	2	0	78	0	0	0	0	0	0	0	152
8:30AM	0	59	0	0	59	0	0	0	10	0	10	0	6	75	4	0	85	0	2	0	0	0	2	0	156
8:45AM	0	68	0	0	68	0	0	0	11	0	11	1	7	102	1	0	110	0	3	0	0	0	3	0	192
9:00AM	0	65	0	0	65	0	0	0	8	0	8	0	10	72	3	0	85	0	1	0	0	0	1	0	159
Total	0	252	0	0	252	0	0	0	43	0	43	1	30	318	10	0	358	0	6	0	0	0	6	0	659
% Approach	0%	100%	0%	0%	_	-	0%	0%	100%	0%	-	-	8.4%	88.8%	2.8%	0%	_	-	100%	0%	0% (0%	_		-
% Total	0%	38.2%	0%	0% 3	38.2%	-	0%	0%	6.5%	0%	6.5%	-	4.6%	48.3%	1.5%	0% !	54.3%	_	0.9%	0% (0% (0%	0.9%		-
PHF	T	0.926	_	_	0.926	-	-	_	0.768	_	0.768	_	0.750	0.779	0.625	_	0.814	_	0.500	_	_		0.500		0.858
Lights	0	219	0	0	219	-	0	0	42	0	42	_	29	286	10	0	325	_	6	0	0	0	6		592
% Lights	0%		0%	0% 8		-	0%	0%	97.7%	0%	97.7%	_	96.7%	89.9%	100%	0% 9	90.8%	_	100%	0% (0% (0%	100%		89.8%
Articulated Trucks	0	25		0	25	-	0	0	0	0	0	_	0	20	0	0	20		0	0	0	0	0		45
% Articulated Trucks	0%	9.9%	0%	0%	9.9%	-	0%	0%	0%	0%	0%	_	0%	6.3%	0%	0%	5.6%		0%	0% (0% (0%	0%		6.8%
Buses and Single-Unit	-																								0.070
Trucks	0	8	0	0	8	-	0	0	1	0	1	-	1	12	0	0	13	-	0	0	0	0	0	-	22
% Buses and Single-Unit																									
Trucks	0%	3.2%	0%	0%	3.2%	-	0%	0%	2.3%	0%	2.3%	-	3.3%	3.8%	0%	0%	3.6%	-	0%	0% (ე% (ე%	0%	-	3.3%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians		-	-	-	-	-	-	-	-	-	-	0%	-	-	-	-	-		-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	100%	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Fri Mar 4, 2022

AM Peak (Mar 04 2022 8:15AM - 9:15 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

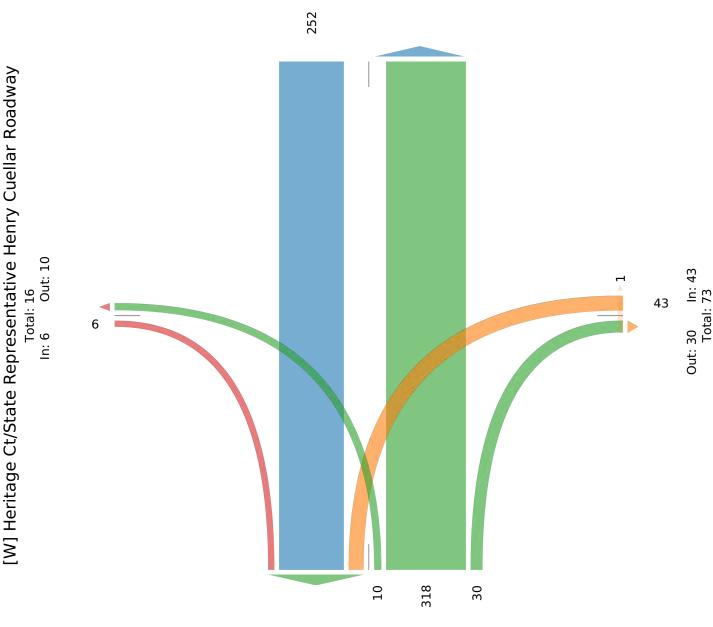
All Movements

ID: 927844, Location: 27.531324, -99.428482

CJ Henson Associates, Inc.

Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US





Out: 301 In: 358 Total: 659 [S] US 59 [E] Heritage Ct/State Representative Henry Cuellar Roadway

Fri Mar 4, 2022

Midday Peak (Mar 04 2022 12:45PM - 1:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927844, Location: 27.531324, -99.428482



8	US 59						Hen	_	uellar Ro		resentati ay		US 59 Northl	oound					Herita Repres Roady Eastbo	senta vay	ative		ıry Cu	ellar	
			, т	T.T.	A T	- J+				T.T.	Δ	Ped*			т	TT	A	n - J*		Т		TT	Δ	D - J*	Total
Time	R	T		U	App F		R		L		App		R	T		U	App							Ped*	
2022-03-04 12:45PM	1	68	_		69	0	0		14	0	14	0	9	95	0	0	104	0	_		0	0	2	0	189
1:00PM	0	88	0	0	88	0	0	0	12	0	12	0	3	93	4	0	100	0	0	0	0	0	0	1	200
1:15PM	0	93	0	0	93	0	0	0	8	0	8	0	10	91	2	0	103	0	0	0	0	0	0	0	204
1:30PM	0	81	. 0	0	81	0	0	0	8	0	8	0	6	95	1	0	102	0	1	0	0	0	1	0	192
Total	1	330	0	0	331	0	0	0	42	0	42	0	28	374	7	0	409	0	3	0	0	0	3	1	785
% Approach	0.3%	99.7%	0%	0%	-	-	0%	0%	100%	0%	_	-	6.8%	91.4%	1.7%	0%	-	-	100%	0%	0% (0%	-	-	-
% Total					12.2%	-	0%	0%	5.4%	0%	5.4%	-	3.6%	47.6%	0.9%	0% 5	52.1%	-	0.4%	0%	0% (0% (0.4%	-	-
PHF	0.250	0.887	_	_	0.890	_	Τ.	_	0.750	_	0.750	_	0.700	0.984	0.438	_	0.983	_	0.375	_	_	- ().375	-	0.962
Lights	1	281	0	0	282	_	0	0	40	0	40		28	334	7	0	369		3	0	0	0	3	_	694
% Lights	100%	85.2%	_	0% 8	35.2%	-	0%	0%	95.2%	0%	95.2%	_	100%	89.3%	100%	0% 9	90.2%	_	100%	0%	0% (0% 1	00%	-	88.4%
Articulated Trucks	0	29			29	_	0		0		0	_	0	27	0	0	27	_	0		0		0	-	56
% Articulated Trucks	0%	8.8%	0%	0%	8.8%	_	0%	0%	0%	0%	0%		0%	7.2%	0%	0%	6.6%		0%	0%	0% (0%	0%	_	7.1%
Buses and Single-Unit	070	0.070	0,0	0,0	0.070		070	0,0	0,0	0,0			070	7.1270	0,0	0,0	0.070		070	0,0	5,0	0,0	0,0		71170
Trucks	0	20	0	0	20	_	0	0	2	0	2	-	0	13	0	0	13	-	0	0	0	0	0	-	35
% Buses and Single-Unit																									
Trucks	0%	6.1%	0%	0%	6.0%	-	0%	0%	4.8%	0%	4.8%	-	0%	3.5%	0%	0%	3.2%	-	0%	0%	0% (0%	0%	-	4.5%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100%	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Fri Mar 4, 2022

Midday Peak (Mar 04 2022 12:45PM - 1:45 PM)

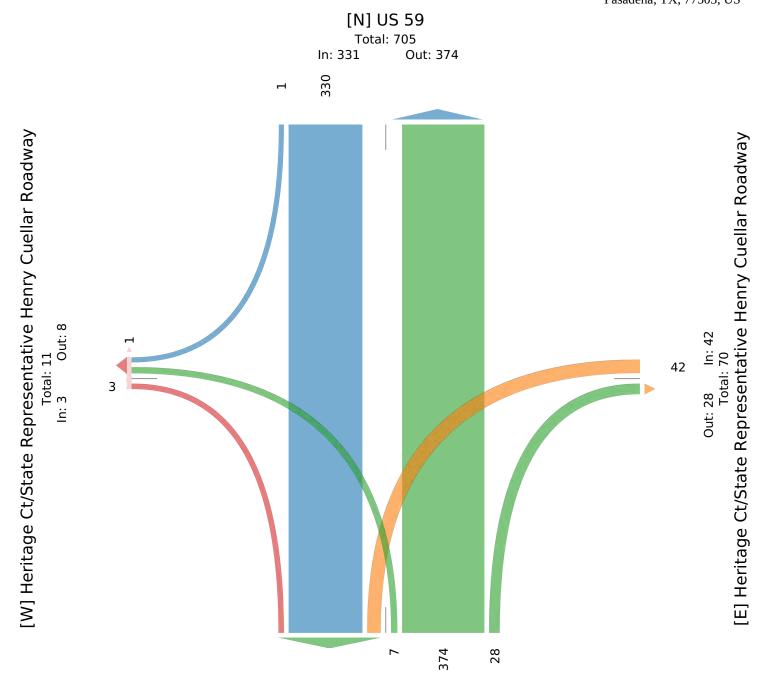
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927844, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US



Out: 375

Total: 784 [S] US 59

In: 409

Fri Mar 4, 2022

PM Peak (Mar 04 2022 7:15PM - 8:15 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927844, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates

5215 Sycamore Ave., Pasadena, TX, 77503, US

	US 5	59 hboun	d				Heritag Henry (Westbo	Cuella			entative	•	US 59 Northl						Heritag Cuellar Eastboo	Roadv		resenta	tive H	enry	
	R		-	T T	A D	_		T	т.	T T	A	D . 14				T T	A T			T	т	T.T.		D . 1*	T
Time		T	L	_	App P	'ea"	R		L		App	Pea*	R	T		U	App I		R		L		App	Pea*	
2022-03-04 7:15PM	0	56	0		56	0	2	1	7	0	10	1	14	149	1	0	164	0	1	1	0	0		- 0	232
7:30PM	0	66	0	_	66	0	1	0	11	_	12	0	10	155		0	172	0	1	0	0	0		0	251
7:45PM	0	61	0	0	61	0	3	0	9	0	12	0	16	161	2	0	179	0	0	0	1	0	1	0	253
8:00PM	0	63	1	0	64	0	0	0	10	0	10	0	14	153	2	0	169	0	1	0	0	1	2	0	245
Total	0	246	1	0	247	0	6	1	37	0	44	1	60	618	6	0	684	0	3	1	1	1	6	0	981
% Approach	0% 9	99.6%	0.4%	0%	-	-	13.6%	2.3%	84.1%	0%	-	-	8.8%	90.4%	0.9%	0%	-	-	50.0%	16.7%	16.7%	16.7%	-	-	-
% Total	0% 2	25.1%	0.1%	0%	25.2%	-	0.6%	0.1%	3.8%	0%	4.5%	-	6.1%	63.0%	0.6%	0% (69.7%	-	0.3%	0.1%	0.1%	0.1%	0.6%	-	-
PHF	-	0.932	0.250	-	0.936	-	0.500	0.250	0.841	- (0.917	-	0.938	0.960	0.750	-	0.955	-	0.750	0.250	0.250	0.250	0.750	-	0.969
Lights	0	227	1	0	228	-	6	1	37	0	44	-	60	597	6	0	663	-	3	1	1	1	6	-	941
% Lights	0% 9	92.3%	100%	0%	92.3%	-	100%	100%	100%	0% :	100%	-	100%	96.6%	100%	0% 9	96.9%	-	100%	100%	100%	100%	100%	-	95.9%
Articulated Trucks	0	14	0	0	14	-	0	0	0	0	0	-	0	14	0	0	14	-	0	0	0	0	0	-	28
% Articulated Trucks	0%	5.7%	0%	0%	5.7%	-	0%	0%	0%	0%	0%	-	0%	2.3%	0%	0%	2.0%	-	0%	0%	0%	0%	0%	-	2.9%
Buses and Single-Unit																									
Trucks	0	5	0	0	5	-	0	0	0	0	0	-	0	7	0	0	7	-	0	0	0	0	0	-	12
% Buses and Single-Unit																									
Trucks	0%	2.0%	0%	0%	2.0%	-	0%	0%	0%	0%	0%	-	0%	1.1%	0%	0%	1.0%	-	0%	0%	0%	0%	0%	_	1.2%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	- 1	.00%	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	0%	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Fri Mar 4, 2022

PM Peak (Mar 04 2022 7:15PM - 8:15 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

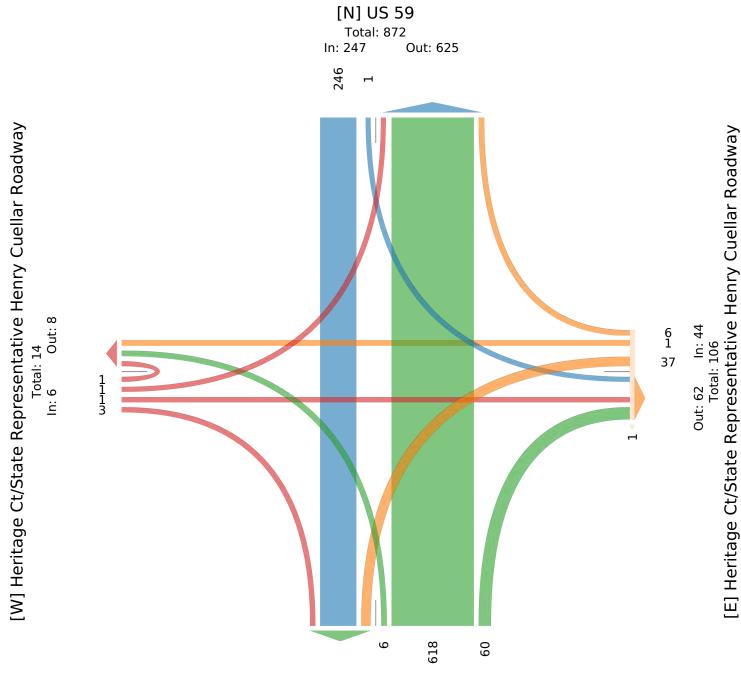
All Movements

ID: 927844, Location: 27.531324, -99.428482

CJ Henson Associates Inc.

Provided by: C. J. Hensch & Associates
Inc.

5215 Sycamore Ave., Pasadena, TX, 77503, US



Out: 286 In: 684 Total: 970 [S] US 59

Sat Mar 5, 2022

Full Length (12 AM-12 AM (+1))

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927845, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

Leg Direction	US 59 Southb	ound				۱ -	, Cuella	State Rep ar Roadv		entative	!	US 59 Northbo	ound					Heritag Henry (Eastbou	Cuellar			entative		
Time	R	Т	L	U	App Ped*	R	Т	L	IJ	Арр	Ped*	R	Т	L	U	App 1	Ped*	R	T	L	IJ	App	Ped*	Int
2022-03-05 12:00AM	1	548	6	0	555 0		0	12	0	13	0	20	48	0	0	68	0	8	0	1	0	9	0	645
1:00AM	0	388	2	0	390 0	0	0	8	0	8	0	9	36	0	0	45	0	11	0	0	0	11	0	454
2:00AM	0	55	0	0	55 0	0	0	1	0	1	0	4	9	0	0	13	0	1	0	1	0	2	0	71
3:00AM	0	19	0	0	19 0	0	0	4	0	4	1	3	14	0	0	17	0	0	0	0	0	0	0	40
4:00AM	1	10	0	0	11 0	0	0	5	0	5	0	7	15	0	0	22	0	1	0	0	0	1	0	39
5:00AM	0	24	0	0	24 0	1	0	9	0	10	0	4	52	0	0	56	0	0	0	0	0	0	0	90
6:00AM	1	36	0	0	37 0	0	0	11	0	11	0	6	78	0	0	84	0	1	0	0	0	1	0	133
7:00AM	0	70	0	0	70 0	0	0	25	0	25	2	12	161	1	0	174	0	0	0	0	0	0	1	269
8:00AM	0	129	3	0	132 0	3	0	33	0	36	3	21	269	8	0	298	0	2	0	0	0	2	0	468
9:00AM	2	140	0	0	142 0	0	0	35	0	35	0	20	381	9	0	410	0	4	0	0	0	4	1	591
10:00AM	2	170	1	0	173 0	0	0	34	0	34	0	18	350	9	0	377	0	5	0	1	0	6	0	590
11:00AM	1	239	0	0	240 0	0	0	36	0	36	0	24	345	5	0	374	0		0	0	0	6	0	656
12:00PM	4	266	1	0	271 0		0	34	0	34	2	33	412	4	1	450	0		0	0	0	7	4	762
1:00PM	1	289	0	0	290 0		0	43	0	47	0	47	425	4	0	476	0		0	0	0	2	0	815
2:00PM	3	286	0	0	289 0		1	32	0	34	0	43	346	5	0	394	0		1	3	0	14	0	731
3:00PM	2	313	2	0	317 0	1	0	32	0	33	0	35	322	7	0	364	0		0	0	0	7	0	721
4:00PM	0	343	1	0	344 0		1	43	0	45	0	49	309	5	0	363	0	11	0	1	0	12	0	764
5:00PM	3	333	0	0	336 0	-	0	40	0	42	0	56	322	16	0	394	0		0	1	0	10	0	782
6:00PM	3	303	3	1	310 0	<u> </u>	0	63	0	67	0	59	439	9	1	508	0	_	0	0	0	5	0	890
7:00PM	6	275	2	0	283 0		0	46	0	50	1	50	519	9	0	578	0		0	2	0	6	0	917
8:00PM	2	284	1	0	287 0		0	37	0	42	0	44	609	5	0	658	0		0	1	0	9	0	996
9:00PM	1	289	1	0	291 0	1	0	28	0	31	0	41	410	4	0	455	0		0	1	0	3	0	780
10:00PM	1	269	2	0	272 0		0	22	0	22	2	30	235	1	0	266	0		0	0	0	9	0	569
11:00PM	3	335	3	0	341 0	_	0	26	0	29	0	23	96	2	0	121	0		0	0	0	5	0	496
Total	37	5413	28	1	5479 0		2	659	0	694	11	658	6202	103	2	6965	0		1	12	0	131	6	13269
% Approach			0.5%	0%				95.0% 0		-	-		89.0%	1.5%	0%	-	-		0.8%			-	-	
% Total	-	40.8%			11.3% -	0.2%	0%	5.0% 0		5.2%			46.7%	0.8%		52.5%	-	0.9%		0.1% (1.0%	-	-
Lights	33	5193	28	1	5255 -	33	2	643	0	678	-	643	6000	91	2	6736	-	106	1	12	0	119	_	12788
% Lights								97.6% 0			-			88.3% 1			-	89.8%					-	96.4%
Articulated Trucks	2	173	0	0	175 -	0	0	1	0	1	-	0	132	0	0	132	-	1	0	0	0	1	-	309
% Articulated Trucks	5.4%	3.2%	0%	0%	3.2% -	0%	0%	0.2% 0	1%	0.1%	-	0%	2.1%	0%	0%	1.9%		0.8%	0%	0% ()%	0.8%	-	2.3%
Buses and Single-Unit Trucks	2	47	0	0	49 -	0	0	15	0	15	_	15	70	12	0	97	_	11	0	0	0	11	_	172
% Buses and Single-Unit	 				-10		- 0	1.0	5	13		13	, 0	14				11			-	- 11	_	
Trucks	5.4%	0.9%	0%	0%	0.9% -	0%	0%	2.3% 0	%	2.2%	-	2.3%	1.1%	11.7%	0%	1.4%	_	9.3%	0%	0% (0%	8.4%	-	1.3%
Pedestrians	-	-	-	-	- 0	-	-	-	-	-	8	-	-	-	-	-	0	-	-	-	-	-	6	
% Pedestrians	-	-	-	-		-	-	-	-	- 7	2.7%	-	-	-	-	-	-	-	-	-	-	- 1	00%	-
Bicycles on Crosswalk	-	-	-	-	- 0	-	-	-	-	-	3	-	-	-	-	-	0	-	-		-	-	0	
% Bicycles on Crosswalk	-	-	-	-		-	-	-	_	- 2	7.3%	-	-	-	_	-	-	-	-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sat Mar 5, 2022

Full Length (12 AM-12 AM (+1))

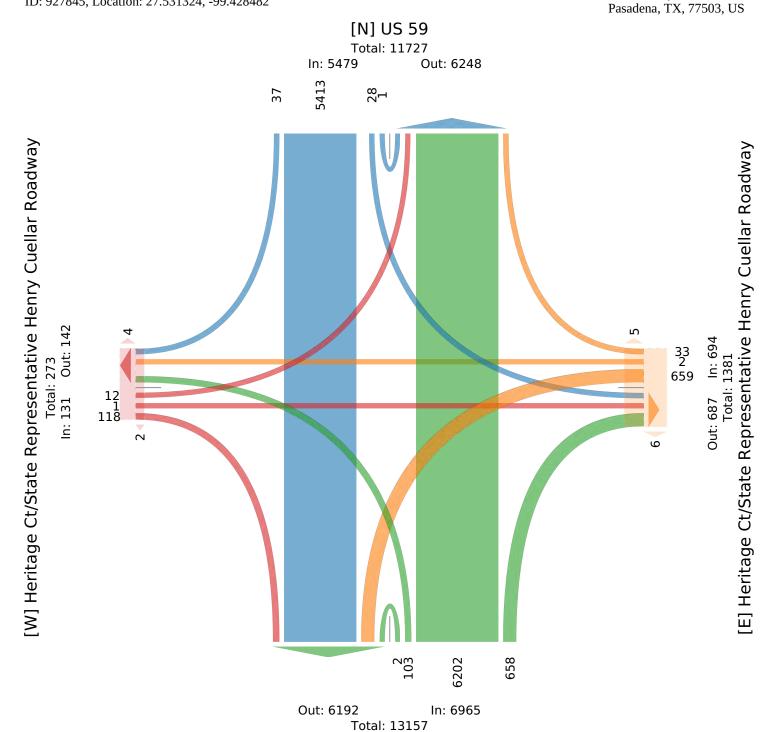
All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927845, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave.,



[S] US 59

Sat Mar 5, 2022

AM Peak (WKND) (12:15 AM - 1:15 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927845, Location: 27.531324, -99.428482



Pasadena, TX, 77503, US

Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave.,

Leg	US	59							/State R lar Road			re	US 59						Heritag Represe Roadwa	entat			ry Cuel	lar	
Direction	Sou	thboun	d				Westb	ound					Northb	ound					Eastbou	,					
Time	R	Т	L	U	App I	Ped*	R	Т	L	U	App	Ped*	R	T	L	U	App P	ed*	R	T	L	U	App	Ped*	Int
2022-03-05 12:15AM	0	116		0	116	0		0	2		2	0		12	0	0	21	0	0		0		0	0	
12:30AM	0	169	4	0	173	0	1	0	2	0	3	0	4	18	0	0	22	0	1	0	0	0	1	0	199
12:45AM	0	147	2	0	149	0	0	0	2	0	2	0	4	12	0	0	16	0	3	0	0	0	3	0	170
1:00AM	0	116		0	118	0	0	0	4		4	0	3	12	0	0	15	0	3		0	0	3	0	_
Total	0	548		0	556	0	1	0	10	0	11	0	20	5/1	0	0	74	0	7	0	0	0	7	0	648
% Approach	Ü			_	-	-	-		90.9%			-	27.0%		_	_			100%	_	_	_			040
% Approach	-						0.2%				1.7%			8.3%					1.1%						
PHF		0.811			0.803	_	0.250	-	0.625		0.688			0.750				_	0.583						0.814
	0			0	551	_			10		11		20		0		71	_			0		6		639
Lights % Lights	Ŭ					_	100%		100%	_				94.4%	_	_		_	85.7%	_	_	_			98.6%
	_					_												_							96.6%
Articulated Trucks	0	3		0	3		_	0		0	0		0		0		3	_			0		0	-	0.004
% Articulated Trucks	0%	0.5%	0%	0%	0.5%		0%	0%	0%	0%	0%	-	0%	5.6%	0% (0%	4.1%	_	0%	J% (0% 0)%	0%		0.9%
Buses and Single-Unit	0	2	0	0	2		١ ,	0	0	0				0	0	0	0			0	0	0			,
Trucks			U	0	2		0	0	0	0	0		0	U	0	U	0	_	1		0	U	1	-	3
% Buses and Single-Unit Trucks	00/	0.40/	00/	00/	0.4%		00/	0%	0%	00/	0%		0%	0%	00/	00/	0%		14.3%	00/	00/ ()0/ 1	4 20/		0.5%
	0%	0.4%			0.4%	-			0%	0%	0%	-											4.5%		
Pedestrians	_	-	-	-	-	0	-	-				0	-	-	-	-	-	0	-	_	-	-	-	0	<u> </u>
% Pedestrians	_	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-			-	-	-	_
Bicycles on Crosswalk	_	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-			-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sat Mar 5, 2022

AM Peak (WKND) (12:15 AM - 1:15 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

[W] Heritage Ct/State Representative Henry Cuellar Roadway

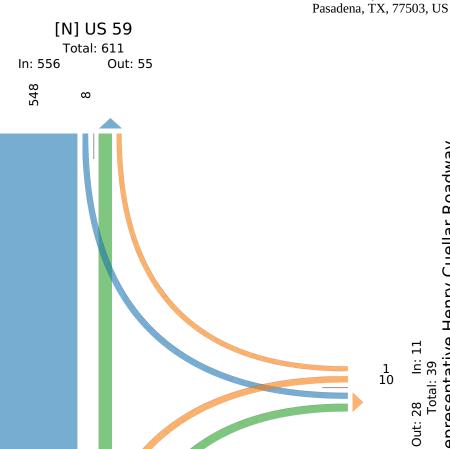
Total:

<u>:</u>

ID: 927845, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave.,



Out: 565 In: 74 Total: 639 [S] US 59

54 20

Sat Mar 5, 2022

Midday Peak (WKND) (12:45 PM - 1:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 927845, Location: 27.531324, -99.428482



Pasadena, TX, 77503, US

Provided by: C. J. Hensch & Associates 5215 Sycamore Ave.,

Leg Direction	US 59							Cue	t/State l llar Roa	•			US 59	ound					Herita Repre Road Eastb	esenta way	ative		nry Cu	ellar	
				T.T.	A F	1.4			-	T T	_											* *		D 14	
Time	R	T		U	App P		_		L		App	Pea*	R	T		U	App 1		_	. T			Арр	Pea*	
2022-03-05 12:45PM	3	78		0	82	0		0	12	0	12	1	7	107	0	1	115	0		. 0		0	4	4	213
1:00PM	0		0	_	69	0		0	12	0	14	0	15	97	0	0	112	0	_ ~			0	0	0	195
1:15PM	1	55	0	0	56	0	0	0	14	0	14	0	8	116	2	0	126	0	C	0	0	0	0	0	196
1:30PM	0	92	0	0	92	0	1	0	10	0	11	0	11	122	1	0	134	0	1	. 0	0	0	1	0	238
Total	4	294	1	0	299	0	3	0	48	0	51	1	41	442	3	1	487	0	5	0	0	0	5	4	842
% Approach	1.3%	98.3%	0.3%	0%	-	-	5.9%	0% 9	94.1% ()%	-	-	8.4%	90.8%	0.6%	0.2%	-	-	100%	0%	0%	0%	-	-	-
% Total	0.5%	34.9%	0.1%	0%:	35.5%	_	0.4%	0%	5.7% ()%	6.1%	-	4.9%	52.5%	0.4%	0.1% 5	57.8%	-	0.6%	0%	0%	0% (0.6%	-	-
PHF	0.333	0.799	0.250	-	0.813	-	0.375	-	0.857	-	0.911	-	0.683	0.906	0.375	0.250	0.909	-	0.313	-	-	- 0).313	-	0.884
Lights	4	282	1	0	287	-	3	0	46	0	49	-	41	429	3	1	474	-	5	0	0	0	5	-	815
% Lights	100%	95.9%	100%	0% 9	96.0%	-	100%	0% 9	95.8% ()% !	96.1%	-	100%	97.1%	100%	100% 9	97.3%	-	100%	0%	0%	0% 1	100%	-	96.8%
Articulated Trucks	0	11	0	0	11	-	0	0	0	0	0	-	0	10	0	0	10	-	0	0	0	0	0	-	21
% Articulated Trucks	0%	3.7%	0%	0%	3.7%	-	0%	0%	0% ()%	0%	-	0%	2.3%	0%	0%	2.1%	-	0%	0%	0%	0%	0%	-	2.5%
Buses and Single-Unit Trucks		1	0	0	1		0	0	2	0	2	_	0	3	0	0	3	_	0	0	0	0	0	_	6
% Buses and Single-Unit																									
Trucks	0%	0.3%	0%	0%	0.3%	-	0% (0%	4.2% ()%	3.9%	-	0%	0.7%	0%	0%	0.6%	-	0%	0%	0%	0%	0%	-	0.7%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0		-	-	-	-	4	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	0%	-	-	-	-	-	-		-	-	-	- 3	100%	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0		-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	- :	100%	-	-	-	-	-	-		-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sat Mar 5, 2022

Midday Peak (WKND) (12:45 PM - 1:45 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

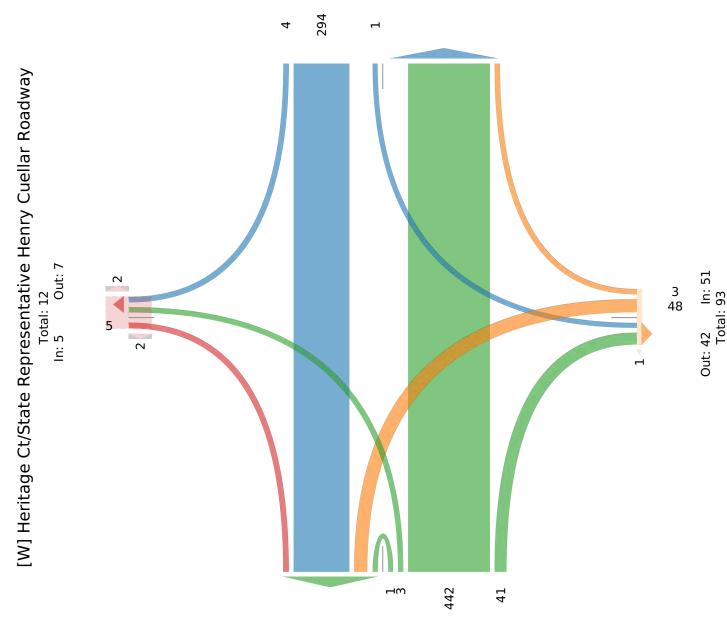
ID: 927845, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US



In: 299 Out: 445



Out: 348 Total: 835

[S] US 59

In: 487

[E] Heritage Ct/State Representative Henry Cuellar Roadway

Sat Mar 5, 2022

PM Peak (WKND) (7:45 PM - 8:45 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles

on Crosswalk)

All Movements

ID: 927845, Location: 27.531324, -99.428482



Provided by: C. J. Hensch & Associates

5215 Sycamore Ave., Pasadena, TX, 77503, US

Leg	US 59							t/State I llar Roa			ive	US 59						Heritag Henry (,			esentativ y	/e	
Direction	Southb	ound				Westb	oun	1				Northb	ound					Eastbou	ınd					
Time	R	Т	L	U	App Ped	* R	T	L	U	App	Ped*	R	T	L	U	App I	Ped*	R	T	L	U	App P	ed*	Int
2022-03-05 7:45PM	3	60	1	0	64	0	0	13	0	13	1	17	150	2	0	169	0	1	0	0	0	1	0	247
8:00PM	1	77	0	0	78 (3	0	10	0	13	0	11	165	1	0	177	0	2	0	1	0	3	0	271
8:15PM	0	77	0	0	77 (0	0	6	0	6	0	11	150	3	0	164	0	0	0	0	0	0	0	247
8:30PM	0	64	. 0	0	64 (1	0	12	0	13	0	13	176	1	0	190	0	0	0	0	0	0	0	267
Total	4	278	1	0	283) 4	0	41	0	45	1	52	641	7	0	700	0	3	0	1	0	4	0	1032
% Approach	1.4%	98.2%	0.4%	0%	-	- 8.9%	0%	91.1% ()%	-	-	7.4%	91.6%	1.0%	0%	-	-	75.0%	0%	25.0%	0%	-	-	-
% Total	0.4%	26.9%	0.1%	0%	27.4%	- 0.4%	0%	4.0% ()%	4.4%	-	5.0%	62.1%	0.7%	0% (57.8%	-	0.3%	0%	0.1%	0%	0.4%	-	-
PHF	0.333	0.903	0.250	-	0.907	- 0.333	-	0.788	-	0.865	-	0.765	0.911	0.583	-	0.921	-	0.375	-	0.250	-	0.333	-	0.952
Lights	3	273	1	0	277	- 4	0	40	0	44	-	51	633	6	0	690	-	2	0	1	0	3	-	1014
% Lights	75.0%	98.2%	100%	0% 9	97.9%	- 100%	0%	97.6% ()% 9	97.8%	-	98.1%	98.8%	85.7%	0% 9	98.6%	-	66.7%	0%	100%	0% 7	75.0%	-	98.3%
Articulated Trucks	1	2	0	0	3	- 0	0	0	0	0	-	0	2	0	0	2	-	0	0	0	0	0	-	5
% Articulated Trucks	25.0%	0.7%	0%	0%	1.1%	- 0%	0%	0% 0)%	0%	-	0%	0.3%	0%	0%	0.3%	-	0%	0%	0%	0%	0%	-	0.5%
Buses and Single-Unit Trucks	0	3	0	0	3	- 0	0	1	0	1	_	1	6	1	0	8	_	1	0	0	0	1		13
% Buses and Single-Unit	1																							
Trucks	0%	1.1%	0%	0%	1.1%	- 0%	0%	2.4% ()%	2.2%	-	1.9%	0.9%	14.3%	0%	1.1%	-	33.3%	0%	0%	0% 2	25.0%	-	1.3%
Pedestrians	-	-	-	-	- () -	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-		-	-	-	-	0%	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	- () -	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-		-	-	-	- 2	100%	-	-	-	-	-	-	-	-	-	-	-	-	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Sat Mar 5, 2022

PM Peak (WKND) (7:45 PM - 8:45 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

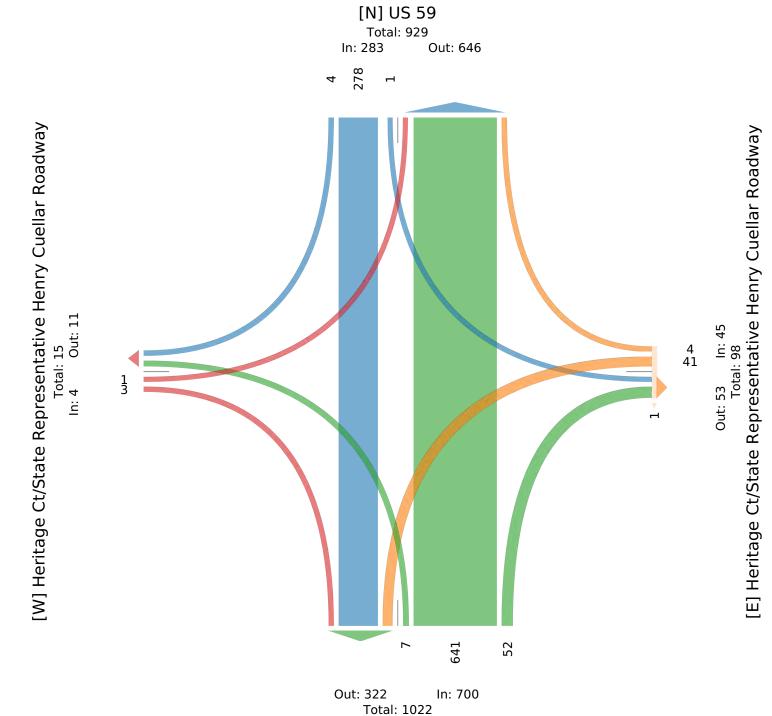
All Movements

ID: 927845, Location: 27.531324, -99.428482

CJ Henson Associates Inc.

Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave..

5215 Sycamore Ave., Pasadena, TX, 77503, US



[S] US 59

8 of 8

Appendix B: Study Area Photos



US 59 at Heritage Court

Looking north towards the Fairground from Heritage Court



Looking south towards City of Laredo from Heritage Court



Truck passing on US 59 at Heritage Court Intersection



Boy Scouts of America Sign at Heritage Court Intersection



US 59 at Wilson Road

Looking at US 59 southbound 59 from Wilson Road



Looking at Wilson Road from US 59



Looking at Webcounty Road and Bridges Entrace from Wilson Rd



Existing Main Entrance (Entrance 2)

Looking at the Main Entrance



Looking towards NE



Looking at SB from the main entrance



Looking at the main entrance and Left Turn queuing



Webb County Fairgrounds Site Pictures

Arena and Restrooms



Vendors and VIP Parking in from of L.I.F.E. Pavilion



Webb County Fair and Expo Sign



Horse Riding



Friday Night Concert





Appendix C: Synchro Report – Existing Conditions



0.7					
EBL	EBR	NBL	NBT	SBT	SBR
				_	- JDIR
	5				1
					1
	-				0
					Free
					None
					-
_					-
_			_	-	
~=			_		85
	-				1
2	8	17	120	195	1
Minor2	P	Maior1		Maior2	
					0
					-
					_
					_
		5.52			_
		_			<u>-</u>
			_		-
			-	-	-
			-	-	-
860	-	-	-	-	-
			-	-	-
	800	942	-	-	-
651	-	-	-	-	-
717	-	-	-	-	-
860	-	-	-	-	-
FB		NB		SB	
		1.1		U	
А					
nt	NBL	NBT	EBLn1	SBT	SBR
		-		-	_
		-		-	-
)	8.9	_	9.7	_	_
	A	_		_	_
	EBL 1 1 0 Stop 0 6, # 0 0 62 1 2 Minor2 278 196 82 5.72 6.62 6.02 3.81 701 730 860 688 651 717 860 EB 9.7 A	EBL EBR 1 5 1 5 0 0 0 0 Stop Stop Stop - None 0 None 0 8, # 0 62 62 1 1 2 8 Minor2 N 278 98 196 82 5.72 7.12 6.62 6.02 3.81 3.91 701 800 730 688 800 651 717 860 EB 9.7 A nt NBL 942 0.018	EBL EBR NBL 1 5 13 1 5 13 0 0 0 0 Stop Stop Free - None 0 - 300 8, # 0 62 62 75 1 1 1 1 2 8 17 Minor2 Major1 278 98 196 196 82 5.72 7.12 5.32 6.62 6.02 3.81 3.91 3.11 701 800 942 730 688 800 942 730 688 800 942 651 717 860 EB NB 9.7 1.1 A nt NBL NBT 942 - 0.018	EBL EBR NBL NBT 1 5 13 90 1 5 13 90 0 0 0 0 0 Stop Stop Free Free - None 0 - 300 - 0	EBL EBR NBL NBT SBT

	≯	•	4	†	↓	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations		#	*	444	44	#	
Traffic Volume (vph)	0	132	539	103	163	12	
Future Volume (vph)	0	132	539	103	163	12	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor		1.00	1.00	0.91	0.95	1.00	
Frt		0.86	1.00	1.00	1.00	0.85	
Flt Protected		1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)		1627	1787	4715	3282	1599	
Flt Permitted		1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)		1627	1787	4715	3282	1599	
Peak-hour factor, PHF	0.93	0.93	0.89	0.89	0.82	0.82	
Adj. Flow (vph)	0	142	606	116	199	15	
RTOR Reduction (vph)	0	57	0	0	0	11	
Lane Group Flow (vph)	0	85	606	116	199	4	
Heavy Vehicles (%)	1%	1%	1%	10%	10%	1%	
Turn Type		Perm	Prot	NA	NA	Perm	
Protected Phases			7	468	8		
Permitted Phases		4				8	
Actuated Green, G (s)		36.0	16.0	60.0	16.0	16.0	
Effective Green, g (s)		36.0	16.0	60.0	16.0	16.0	
Actuated g/C Ratio		0.60	0.27	1.00	0.27	0.27	
Clearance Time (s)		4.0	4.0		4.0	4.0	
Lane Grp Cap (vph)		976	476	4715	875	426	
v/s Ratio Prot			c0.34	c0.02	c0.06		
v/s Ratio Perm		0.05				0.00	
v/c Ratio		0.09	1.27	0.02	0.23	0.01	
Uniform Delay, d1		5.1	22.0	0.0	17.2	16.2	
Progression Factor		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2		0.2	138.5	0.0	0.6	0.0	
Delay (s)		5.2	160.5	0.0	17.8	16.2	
Level of Service		Α	F	Α	В	В	
Approach Delay (s)	5.2			134.7	17.7		
Approach LOS	А			F	В		
Intersection Summary							
HCM 2000 Control Delay			94.4	Н	CM 2000	Level of Service	
HCM 2000 Volume to Capacit	ty ratio		0.51				
Actuated Cycle Length (s)			60.0	S	um of lost	time (s)	•
Intersection Capacity Utilization	on		41.0%	IC	CU Level c	of Service	
Analysis Period (min)			15				

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			4		*	^ ^		*	^ ^	
Traffic Vol, veh/h	1	1	3	3	1	3	2	639	5	1	294	1
Future Vol, veh/h	1	1	3	3	1	3	2	639	5	1	294	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	38	38	38	75	75	75	91	91	91	89	89	89
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	8	4	1	4	2	702	5	1	330	1
Major/Minor	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	618	1044	166	845	1042	354	331	0	0	707	0	0
Stage 1	333	333	-	709	709	-	-	-	-	-	-	-
Stage 2	285	711	-	136	333	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	428	229	725	317	230	551	816	-	-	544	-	-
Stage 1	570	645	-	318	438	-	-	-	-	-	-	-
Stage 2	643	437	-	787	645	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	422	228	725	310	229	551	816	-	-	544	-	-
Mov Cap-2 Maneuver	422	228	-	310	229	-	-	-	-	-	-	-
Stage 1	569	644	-	317	437	-	-	-	-	-	-	-
Stage 2	635	436	-	774	644	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.1			15.3			0			0		
HCM LOS	В			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1\	NBI n1	SBL	SBT	SBR			
Capacity (veh/h)		816			459	359	544					
HCM Lane V/C Ratio		0.003	_	_	0.029	0.026	0.002	_	_			
HCM Control Delay (s)		9.4	_	_	13.1	15.3	11.6	_	_			
HCM Lane LOS		3. 4	_	_	В	C	В	_	_			
HCM 95th %tile Q(veh))	0	_	-	0.1	0.1	0	_	_			
7000 4(1011)					5.7	J. 7						

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			43-		*	*		*	444	
Traffic Vol, veh/h	1	1	3	41	1	4	7	641	52	1	295	4
Future Vol, veh/h	1	1	3	41	1	4	7	641	52	1	295	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	33	33	33	86	86	86	92	92	92	91	91	91
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	9	48	1	5	8	697	57	1	324	4
Major/Minor I	Minor2			Minor1			Major1		_ N	Major2		
Conflicting Flow All	623	1098	164	875	1072	377	328	0	0	754	0	0
Stage 1	328	328	-	742	742	-	-	-	-	-	-	-
Stage 2	295	770	-	133	330	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	425	213	727	304	221	532	819	_	-	517	-	-
Stage 1	574	648	-	302	423	-	-	-	-	-	-	-
Stage 2	634	411	-	790	647	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	416	210	727	294	218	532	819	-	-	517	-	-
Mov Cap-2 Maneuver	416	210	-	294	218	-	-	-	-	-	-	-
Stage 1	568	647	-	299	419	-	-	-	-	-	-	-
Stage 2	621	407	-	775	646	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.4			19.4			0.1			0		
HCM LOS	В			С								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		819	-	-	443	304	517	-	-			
HCM Lane V/C Ratio		0.009	-	-		0.176	0.002	-	-			
HCM Control Delay (s)		9.4	-	-	13.4	19.4	12	-	-			
HCM Lane LOS		Α	-	-	В	С	В	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.1	0.6	0	-	-			
-(-)												

Intersection						
Int Delay, s/veh	3.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	- 14		- 7	**	ቀቀሴ	
Traffic Vol, veh/h	1	15	4	30	18	1
Future Vol, veh/h	1	15	4	30	18	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	300	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	36	36	81	81	56	56
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	3	42	5	37	32	2
Major/Minor	Minor2		Acior1		Major2	
•			Major1		Major2	
Conflicting Flow All	58	17	34	0	-	0
Stage 1	33	-	-	-	-	-
Stage 2	25	-		-	-	-
Critical Hdwy	5.72	7.12	5.32	-	-	-
Critical Hdwy Stg 1	6.62	-	-	-	-	-
Critical Hdwy Stg 2	6.02	-	-	-	-	-
Follow-up Hdwy	3.81	3.91	3.11	-	-	-
Pot Cap-1 Maneuver	888	899	1117	-	-	-
Stage 1	905	-	-	-	-	-
Stage 2	918	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	884	899	1117	-	-	-
Mov Cap-2 Maneuver	817	-	-	-	-	-
Stage 1	901	-	-	-	-	-
Stage 2	918	-	-	-	-	-
5 H. 95 =						
A			ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	9.2		1		0	
HCM LOS	Α					
Minor Lane/Major Mvmt	•	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)		1117	-	893	- 051	- ODIT
HCM Lane V/C Ratio		0.004	_	0.05	_	_
HCM Control Delay (s)		8.2	_	9.2	_	
HCM Lane LOS		6.2 A	-	9.2 A	_	_
HCM 95th %tile Q(veh)		0	_	0.2		_
HOW SOUL WILLE (Ven)		U	_	0.2	_	_

Intersection						
Int Delay, s/veh	16.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**	LDIX	ħ		*	ODIT
Traffic Vol, veh/h	7	527	21	TTT 28	32	1
Future Vol, veh/h	7	527	21	28	32	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Siop	None	-	None	-	None
Storage Length	0	-	300	INUITE	_	None
Veh in Median Storage	-	_	300	0	0	
Grade, %	, # 0	-	_	0	0	_
•	83				46	
Peak Hour Factor		83	64	64		46
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	8	635	33	44	70	2
Major/Minor	Minor2	N	//ajor1		Major2	
Conflicting Flow All	155	36	72	0	-	0
Stage 1	71	-	_	_	_	_
Stage 2	84	_	_	_	_	_
Critical Hdwy	5.72	7.12	5.32	_	_	_
Critical Hdwy Stg 1	6.62	7.12	0.02	_	_	_
Critical Hdwy Stg 2	6.02	_	_			
Follow-up Hdwy	3.81	3.91	3.11	-	_	_
	801	874	1073	-		-
Pot Cap-1 Maneuver	861	-				
Stage 1		-	-	-	-	-
Stage 2	858	-	-	-	_	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	776	874	1073	-	-	-
Mov Cap-2 Maneuver	739	-	-	-	-	-
Stage 1	834	-	-	-	-	-
Stage 2	858	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	19.8		3.6		0	
HCM LOS	19.0 C		3.0		U	
HCIVI LOS	U					
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1073	-	872	-	-
HCM Lane V/C Ratio		0.031	-		-	-
HCM Control Delay (s)		8.5	-	19.8	-	-
HCM Lane LOS		A	-	С	-	-
HCM 95th %tile Q(veh)		0.1	-	6.8	-	-
rioni ooni /onio Q(ven)		0.1		0.0		

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			43-		*	^ ^		*	444	
Traffic Vol. veh/h	1	1	1	1	1	1	1	48	1	1	559	1
Future Vol. veh/h	1	1	1	1	1	1	1	48	1	1	559	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-		None			None	-	-	None	-	-	None
Storage Length	-	-	-	_	-	-	300	-	-	300	_	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	25	25	25	50	50	50	67	67	67	79	79	79
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	4	4	4	2	2	2	1	72	1	1	708	1
Major/Minor I	Minor2			Minor1		I	Major1		1	Major2		
Conflicting Flow All	743	786	355	362	786	37	709	0	0	73	0	0
Stage 1	711	711	-	75	75	-	-	-	-	-	-	-
Stage 2	32	75	-	287	711	-	_	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	_	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	363	325	550	596	325	873	543	-	-	1072	-	-
Stage 1	317	437	-	844	834	-	-	-	-	-	-	-
Stage 2	905	834	-	641	437	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	360	324	550	585	324	873	543	-	-	1072	-	-
Mov Cap-2 Maneuver	360	324	-	585	324	-	-	-	-	-	-	-
Stage 1	316	437	-	842	832	-	-	-	-	-	-	-
Stage 2	899	832	-	630	437	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	14.5			12.2			0.2			0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		543	-	-	391	505	1072	-	-			
HCM Lane V/C Ratio		0.003	-	-	0.031	0.012	0.001	-	-			
HCM Control Delay (s)		11.6	-	-	14.5	12.2	8.4	-	-			
HCM Lane LOS		В	-	-	В	В	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.1	0	0	-	-			

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			43-		*	*		*	^	
Traffic Vol, veh/h	1	1	7	12	1	1	1	47	20	6	554	1
Future Vol, veh/h	1	1	7	12	1	1	1	47	20	6	554	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	40	40	40	54	54	54	80	80	80	84	84	84
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	18	22	2	2	1	59	25	7	660	1
Major/Minor N	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	702	761	331	354	749	42	661	0	0	84	0	0
Stage 1	675	675	-	74	74	-	-	-	-	-	-	-
Stage 2	27	86	-	280	675	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52		7.32	5.52		-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	383	336	570	603	341	867	572	-	-	1060	-	-
Stage 1	335	454	-	845	835	-	-	-	-	-	-	-
Stage 2	911	825	-	647	454	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	378	333	570	578	338	867	572	-	-	1060	-	-
Mov Cap-2 Maneuver	378	333	-	578	338	-	-	-	-	-	-	-
Stage 1	334	451	-	843	833	-	-	-	-	-	-	-
Stage 2	905	823	-	620	451	-	-	-	-	-	-	-
g- =												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.5			11.7			0.2			0.1		
HCM LOS	В.			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		572	_	-	502	563	1060	-	-			
HCM Lane V/C Ratio		0.002	-	-	0.045	0.046	0.007	-	-			
HCM Control Delay (s)		11.3	_	-	12.5	11.7	8.4	-	-			
HCM Lane LOS		В	_	-	В.	В	A	-	-			
HCM 95th %tile Q(veh)		0	_	-	0.1	0.1	0	-	-			
					٠.,	J.,						

Appendix D: SimTraffic Report – Existing Conditions



Intersection: 4: US 59 & Entrance 1

Movement	EB	NB
Directions Served	LR	L
Maximum Queue (ft)	31	2
Average Queue (ft)	6	0
95th Queue (ft)	26	2
Link Distance (ft)	1113	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		300
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 6: US 59 & Entrance 2

Movement	EB	NB	NB	NB	SB	SB	SB
Directions Served	R	L	T	Т	Т	T	R
Maximum Queue (ft)	28	325	839	718	90	86	31
Average Queue (ft)	1	316	646	283	42	30	6
95th Queue (ft)	15	363	1055	813	73	73	24
Link Distance (ft)	1029		718	718	1366	1366	1366
Upstream Blk Time (%)			51	0			
Queuing Penalty (veh)			114	1			
Storage Bay Dist (ft)		300					
Storage Blk Time (%)		75	0				
Queuing Penalty (veh)		27	3				

Intersection: 8: US 59 & Entrance to Webb County Road and Bridges/Wilson Rd

Movement	EB	WB	B1	NB	NB	NB	SB		
Directions Served	LTR	LTR	Т	L	Т	Т	L		
Maximum Queue (ft)	46	57	6	8	456	382	9		
Average Queue (ft)	10	13	1	0	147	85	1		
95th Queue (ft)	37	48	9	4	486	360	6		
Link Distance (ft)	1056	40	1289		653	653			
Upstream Blk Time (%)		17			2	0			
Queuing Penalty (veh)		0			4	0			
Storage Bay Dist (ft)				300			300		
Storage Blk Time (%)					17				
Queuing Penalty (veh)					0				

Intersection: 12: US 59 & State Rep. Henry Cuellar Rdwy/Heritage Ct

Movement	EB	WB	NB	NB	NB	SB
Directions Served	LTR	LTR	L	Т	T	L
Maximum Queue (ft)	36	85	22	21	15	4
Average Queue (ft)	7	33	2	2	1	0
95th Queue (ft)	29	65	13	31	22	3
Link Distance (ft)	980	198		1856	1856	
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			300			300
Storage Blk Time (%)						
Queuing Penalty (veh)						

Zone Summary

Zone wide Queuing Penalty: 149

Intersection: 4: US 59 & Entrance 1

Movement	EB
Directions Served	LR
Maximum Queue (ft)	38
Average Queue (ft)	15
95th Queue (ft)	39
Link Distance (ft)	1113
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 6: US 59 & Entrance 2

Movement	EB	NB
Directions Served	LR	L
Maximum Queue (ft)	196	16
Average Queue (ft)	87	1
95th Queue (ft)	147	10
Link Distance (ft)	1029	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		300
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 8: US 59 & Entrance to Webb County Road and Bridges/Wilson Rd

Movement	EB	WB	NB	SB	
Directions Served	LTR	LTR	L	L	
Maximum Queue (ft)	29	25	3	2	
Average Queue (ft)	4	4	0	0	
95th Queue (ft)	19	19	2	2	
Link Distance (ft)	1056	40			
Upstream Blk Time (%)		0			
Queuing Penalty (veh)		0			
Storage Bay Dist (ft)			300	300	
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 12: US 59 & State Rep. Henry Cuellar Rdwy/Heritage Ct

Movement	EB	WB	NB	SB	
Directions Served	LTR	LTR	L	L	
Maximum Queue (ft)	39	30	7	5	
Average Queue (ft)	10	13	0	0	
95th Queue (ft)	34	37	4	3	
Link Distance (ft)	980	198			
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)			300	300	
Storage Blk Time (%)					
Queuing Penalty (veh)					

Zone Summary

Zone wide Queuing Penalty: 0

Appendix E: Sight Distance at Wilson Road and Heritage Court







FAIRGROUNDS TRAFFIC STUDY

Sight Distance Of Wilson Road

Appendix F: Synchro Report – 2025 Build Conditions



	•	•	4	†	ļ	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations			75.75	*	•	#	
Traffic Volume (vph)	0	0	563	120	189	113	
Future Volume (vph)	0	0	563	120	189	113	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)			4.0	4.0	4.0	4.0	
Lane Util. Factor			0.97	1.00	1.00	1.00	
Frt			1.00	1.00	1.00	0.85	
Flt Protected			0.95	1.00	1.00	1.00	
Satd. Flow (prot)			3467	1727	1727	1599	
Flt Permitted			0.95	1.00	1.00	1.00	
Satd. Flow (perm)			3467	1727	1727	1599	
Peak-hour factor, PHF	0.93	0.93	0.82	0.82	0.82	0.82	
Adj. Flow (vph)	0	0	687	146	230	138	
RTOR Reduction (vph)	0	0	0	0	0	101	
Lane Group Flow (vph)	0	0	687	146	230	37	
Heavy Vehicles (%)	1%	1%	1%	10%	10%	1%	
Turn Type			Prot	NA	NA	Perm	
Protected Phases			7	468	8		
Permitted Phases						8	
Actuated Green, G (s)			16.0	60.0	16.0	16.0	
Effective Green, g (s)			16.0	60.0	16.0	16.0	
Actuated g/C Ratio			0.27	1.00	0.27	0.27	
Clearance Time (s)			4.0		4.0	4.0	
Lane Grp Cap (vph)			924	1727	460	426	
v/s Ratio Prot			c0.20	c0.08	c0.13		
v/s Ratio Perm						0.02	
v/c Ratio			0.74	0.08	0.50	0.09	
Uniform Delay, d1			20.1	0.0	18.6	16.5	
Progression Factor			1.00	1.00	1.00	1.00	
Incremental Delay, d2			5.4	0.1	3.8	0.4	
Delay (s)			25.5	0.1	22.5	16.9	
Level of Service			С	Α	С	В	
Approach Delay (s)	0.0			21.1	20.4		
Approach LOS	Α			С	С		
Intersection Summary							
HCM 2000 Control Delay			20.9	H	CM 2000	Level of Service	
HCM 2000 Volume to Capac	city ratio		0.45				
Actuated Cycle Length (s)			60.0		um of lost		
Intersection Capacity Utilizat	tion		32.7%	IC	U Level c	of Service	
Analysis Period (min)			15				

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		77		*	•	
Traffic Vol, veh/h	0	455	0	682	189	0
Future Vol, veh/h	0	455	0	682	189	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Free	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	ŧ 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	62	62	82	82	82	82
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	0	734	0	832	230	0
						-
	inor2	N	//ajor1		Major2	
Conflicting Flow All	-	-	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-
Pot Cap-1 Maneuver	0	0	0	-	_	0
Stage 1	0	0	0	-	-	0
Stage 2	0	0	0	_	_	0
Platoon blocked, %	Ū	· ·	Ū	_	_	•
Mov Cap-1 Maneuver	_	_	_	_	_	_
Mov Cap-1 Maneuver	_	_	_	-	-	_
Stage 1	-	-	-	-	_	
Stage 2	-	-	-	-	-	-
Stage 2		-		-	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Ā					
	, ,					
Minor Lane/Major Mvmt		NBT E	EBLn1	EBLn2	SBT	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		-	-	-	-	
LIOM Ossal Dalass (s)		_	0	0	_	
HCM Control Delay (s)			U	U		
HCM Control Delay (s) HCM Lane LOS		-	A	A	-	

	•	*	•	†	↓	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			75	•	ተ ቀኄ	
Traffic Volume (veh/h)	0	0	450	682	644	1
Future Volume (Veh/h)	0	0	450	682	644	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.62	0.62	0.82	0.82	0.82	0.82
Hourly flow rate (vph)	0	0	549	832	785	1
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				TWLTL	TWLTL	
Median storage veh)				2	2	
Upstream signal (ft)					1177	
pX, platoon unblocked						
vC, conflicting volume	2716	262	786			
vC1, stage 1 conf vol	786					
vC2, stage 2 conf vol	1930					
vCu, unblocked vol	2716	262	786			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	34			
cM capacity (veh/h)	34	739	835			
, , ,	_			CD 4	CD 0	CD 0
Direction, Lane #	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	274	274	832	314	314	158
Volume Left	274	274	0	0	0	0
Volume Right	0	0	0	0	0	1
cSH	835	835	1700	1700	1700	1700
Volume to Capacity	0.66	0.66	0.49	0.18	0.18	0.09
Queue Length 95th (ft)	126	126	0	0	0	0
Control Delay (s)	17.2	17.2	0.0	0.0	0.0	0.0
Lane LOS	С	С				
Approach Delay (s)	6.8			0.0		
Approach LOS						
Intersection Summary						
Average Delay			4.4			
Intersection Capacity Utiliz	zation		39.2%	10	CU Level o	f Service
Analysis Period (min)			15			

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL	T T	INDL		*	ODIN
Traffic Vol, veh/h	0		0	TTT 1132	77 L	1
Future Vol, veh/h	0	1	0	1132	644	1
Conflicting Peds, #/hr	0	0	0	0	044	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	_	0	_	-	_	-
Veh in Median Storage,		-	_	0	0	_
Grade, %	# 0 0	_	_	0	0	_
Peak Hour Factor	93	93	89	89	82	82
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	0	1	0	1272	785	1
Major/Minor N	/linor2	ľ	Major1		Major2	
Conflicting Flow All	-	393		0		0
Stage 1	-	-	-	-	-	-
Stage 2	-	_	_	_	_	_
Critical Hdwy	_	7.12	_	_	_	_
Critical Hdwy Stg 1	_		_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	-	3.91	_	_	_	_
Pot Cap-1 Maneuver	0	520	0	_	_	_
Stage 1	0	520	0	_	_	_
Stage 2	0	_	0	_	_	_
Platoon blocked, %	U	_	U	-	_	_
Mov Cap-1 Maneuver	_	520	_	-		_
		520	-		_	_
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	_	_	-	-	_	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	11.9		0		0	
HCM LOS	В		U		U	
TIOM EGG						
Minor Lane/Major Mvmt		NBT I	EBLn1	SBT	SBR	
Capacity (veh/h)		-	520	-	-	
HCM Lane V/C Ratio		-	0.002	-	-	
HCM Control Delay (s)		-	11.9	-	-	
HCM Lane LOS		-	В	-	-	
HCM 95th %tile Q(veh)		-	0	-	-	

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	^ ^		¥	^ ^	
Traffic Vol, veh/h	1	1	1	3	1	3	1	1129	6	1	644	1
Future Vol, veh/h	1	1	1	3	1	3	1	1129	6	1	644	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	38	38	38	75	75	75	91	91	91	89	89	89
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	3	4	1	4	1	1241	7	1	724	1
Major/Minor N	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	1226	1977	363	1540	1974	624	725	0	0	1248	0	0
Stage 1	727	727	-	1247	1247	-	-	-	-	-	-	-
Stage 2	499	1250	-	293	727	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	189	62	543	123	62	369	534	-	-	299	-	-
Stage 1	309	430	-	135	245	-	-	-	-	-	-	-
Stage 2	479	245	-	636	430	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	183	62	543	118	62	369	534	-	-	299	-	-
Mov Cap-2 Maneuver	183	62	-	118	62	-	-	-	-	-	-	-
Stage 1	308	429	-	135	245	_	-	-	-	-	-	-
Stage 2	470	245	-	627	429	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	35			32.3			0			0		
HCM LOS	E			D D								
	_											
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1\	NBI n1	SBL	SBT	SBR			
Capacity (veh/h)		534	-	-	128	141	299		- ODIT			
HCM Lane V/C Ratio		0.002	_	<u>-</u>	0.062	0.066	0.004	-	-			
HCM Control Delay (s)		11.8	_	_	35	32.3	17.1	_	_			
HCM Lane LOS		В	_	_	E	52.5 D	17.1 C	_	_			
HCM 95th %tile Q(veh)		0	_	_	0.2	0.2	0	_				
TOTAL COULT FOUND Q(VOIT)		J			0.2	0.2	3					

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43-			43		*	*		*	444	
Traffic Vol, veh/h	1	1	3	47	1	5	8	1129	59	1	642	5
Future Vol, veh/h	1	1	3	47	1	5	8	1129	59	1	642	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	33	33	33	86	86	86	92	92	92	91	91	91
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	9	55	1	6	9	1227	64	1	705	5
Major/Minor	Minor2			Minor1			Major1		ľ	Major2		
Conflicting Flow All	1219	2019	355	1563	1989	646	710	0	0	1291	0	0
Stage 1	710	710	-	1277	1277	-	-	-	-	-	-	-
Stage 2	509	1309	-	286	712	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	191	58	550	119	61	357	542	-	-	285	-	-
Stage 1	317	437	-	128	237	-	-	-	-	-	-	-
Stage 2	473	229	-	642	436	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	182	57	550	111	60	357	542	-	-	285	-	-
Mov Cap-2 Maneuver	182	57	-	111	60	-	-	-	-	-	-	-
Stage 1	312	435	-	126	233	-	-	-	-	-	-	-
Stage 2	455	225	-	625	434	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	27.5			65.8			0.1			0		
HCM LOS	D			F								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		542	-	-	175	117	285	-	-			
HCM Lane V/C Ratio		0.016	-	-	0.087	0.527	0.004	-	-			
HCM Control Delay (s)		11.8	-	-	27.5	65.8	17.7	-	-			
HCM Lane LOS		В	-	-	D	F	С	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.3	2.5	0	-	-			
,												

	۶	•	4	†	ļ	4		
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	*	11	*	•	*	#		
Traffic Volume (vph)	455	2500	24	38	25	1		
Future Volume (vph)	455	2500	24	38	25	1		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0		
Lane Util. Factor	1.00	0.88	1.00	1.00	1.00	1.00		
Frt	1.00	0.85	1.00	1.00	1.00	0.85		
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00		
Satd. Flow (prot)	1787	2814	1787	1727	1727	1599		
Flt Permitted	0.95	1.00	0.59	1.00	1.00	1.00		
Satd. Flow (perm)	1787	2814	1101	1727	1727	1599		
Peak-hour factor, PHF	0.83	0.83	0.64	0.64	0.64	0.64		
Adj. Flow (vph)	548	3012	38	59	39	2		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	548	3012	38	59	39	2		
Heavy Vehicles (%)	1%	1%	1%	10%	10%	1%		
Turn Type	Prot	Free	custom	NA	NA	Free		
Protected Phases	5		7	468	8			
Permitted Phases		Free	4			Free		
Actuated Green, G (s)	47.0	100.0	24.0	45.0	16.0	100.0		
Effective Green, g (s)	47.0	100.0	24.0	45.0	16.0	100.0		
Actuated g/C Ratio	0.47	1.00	0.24	0.45	0.16	1.00		
Clearance Time (s)	4.0		4.0		4.0			
Lane Grp Cap (vph)	839	2814	291	777	276	1599		
v/s Ratio Prot	0.31		0.01	0.03	0.02			
v/s Ratio Perm		c1.07	0.03			0.00		
v/c Ratio	0.65	1.07	0.13	0.08	0.14	0.00		
Uniform Delay, d1	20.3	50.0	29.6	15.7	36.1	0.0		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	3.9	39.5	0.9	0.2	1.1	0.0		
Delay (s)	24.2	89.5	30.5	15.9	37.2	0.0		
Level of Service	С	F	С	В	D	Α		
Approach Delay (s)	79.4			21.6	35.4			
Approach LOS	Е			С	D			
Intersection Summary								
HCM 2000 Control Delay			77.4	Н	CM 2000	Level of Service	!	
HCM 2000 Volume to Capacit	y ratio		1.27					
Actuated Cycle Length (s)			100.0	St	ım of lost	time (s)		
Intersection Capacity Utilization	n		39.9%	IC	U Level c	of Service		
Analysis Period (min)			15					

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL		INDL		*	JUIN
Traffic Vol, veh/h	0		0	TT	2525	1
Future Vol. veh/h	0	1	0	66	2525	1
,	0	0	0	00	2525	0
Conflicting Peds, #/hr						
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	36	36	64	64	64	64
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	0	3	0	103	3945	2
Major/Minor N	/linor2		Major1		Major2	
•			viajor i -	0	wajorz -	0
Conflicting Flow All	-	1974				0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.12	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.91	-	-	-	-
Pot Cap-1 Maneuver	0	45	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	45	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	_	_	-	-	-
Stage 2	_	_	_	_	_	_
Olage 2						
Approach	EB		NB		SB	
HCM Control Delay, s	90.2		0		0	
HCM LOS	F					
		Non	-DI (05-	055	
Minor Lane/Major Mvmt			EBLn1	SBT	SBR	
Capacity (veh/h)		-	45	-	-	
HCM Lane V/C Ratio		-	0.062	-	-	
HCM Control Delay (s)		-	90.2	-	-	
HCM Lane LOS		-	F	-	-	
HCM 95th %tile Q(veh)		-	0.2	-	-	

Int Delay, s/veh	Intersection						
Lane Configurations		0					
Lane Configurations	Movement	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Vol, veh/h 0 1591 0 66 2525 0 Future Vol, veh/h 0 1591 0 66 2525 0 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free None Call Call Call				TIDE			OD. N
Future Vol, veh/h Conflicting Peds, #/hr O O O O O O O O O O O O O O O O O O O		0		0			0
Conflicting Peds, #/hr							0
Sign Control Stop Stop Free Rone None Poll Poll Poll All 1 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0</td>							0
RT Channelized - Free - None - None Storage Length - 0 0 0 0							
Storage Length							
Veh in Median Storage, # 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 - - 0 0 0 - 0 0 0 - - 0 - 0 0 0 - <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td>							-
Grade, % 0 0 0 0 0 0 Peak Hour Factor 36 36 64 64 64 64 64 Heavy Vehicles, % 1 1 1 1 10 10 1 Mwmt Flow 0 4419 0 103 3945 0 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 0 0 - 0 Stage 1 0 - 0 - 0 Stage 2 0 - 0 - 0 Stage 2 0 - 0 - 0 Stage 1 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 - 0 Stage 1 - 0 - 0 - 0 Stage 2 - 0 0 0 0 - 0 - 0 Stage 1 - 0 - 0 - 0 Stage 2 - 0 0 0 0 - 0 - 0 Stage 1 - 0 - 0 Stage 1 - 0 - 0 - 0 Stage 2 - 0 0 0 0 - 0 - 0 Stage 1 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 - 0 - 0 Stage 2 - 0 - 0 Stage 2 - 0 - 0 - 0 Stage 2 - 0 Stage 2 - 0 - 0 - 0 Stage 2 - 0 Stage 2			-				_
Peak Hour Factor 36 36 64 64 64 64 Heavy Vehicles, % 1 1 1 10 10 1 Mwmt Flow 0 4419 0 103 3945 0 Major/Minor Minor2 Major1 Major2 Conflicting Flow All - - 0 0 Stage 1 - - 0 0 Stage 2 - - - - Critical Hdwy - - - - Critical Hdwy Stg 1 - - - - Critical Hdwy Stg 2 - - - - Follow-up Hdwy - - - - Pot Cap-1 Maneuver 0 0 - - 0 Stage 1 0 0 0 - - 0 Mov Cap-1 Maneuver - - - - - - Mov Cap-2 Maneuver							_
Heavy Vehicles, %	•	-					
Momental Flow 0 4419 0 103 3945 0 Major/Minor Minor2 Major1 Major2 Conflicting Flow All - - 0 0 Stage 1 - - - - Stage 2 - - - - Critical Hdwy - - - - Critical Hdwy Stg 1 - - - - Critical Hdwy Stg 2 - - - - - Follow-up Hdwy - <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Major/Minor Minor2 Major1 Major2 Conflicting Flow All - - 0 0 Stage 1 - - - - Stage 2 - - - - Critical Hdwy - - - - Critical Hdwy Stg 1 - - - - Critical Hdwy Stg 2 - - - - Follow-up Hdwy - - - - - Pot Cap-1 Maneuver 0 0 0 - - 0 Stage 1 0 0 0 - - 0 Platoon blocked, % - - - - - Mov Cap-1 Maneuver - - - - - Mov Cap-2 Maneuver - - - - - Stage 1 - - - - - Stage 2 - - -			-				
Conflicting Flow All 0 - 0 - 0 Stage 1	WWIII FIOW	U	4419	U	103	3945	U
Conflicting Flow All 0 - 0 - 0 Stage 1							
Stage 1 - </td <td>Major/Minor M</td> <td>inor2</td> <td>1</td> <td>Major1</td> <td></td> <td>Major2</td> <td></td>	Major/Minor M	inor2	1	Major1		Major2	
Stage 1 - </td <td></td> <td></td> <td>-</td> <td>-</td> <td>0</td> <td>-</td> <td>0</td>			-	-	0	-	0
Stage 2 - </td <td></td> <td>-</td> <td></td> <td>-</td> <td>-</td> <td>-</td> <td>-</td>		-		-	-	-	-
Critical Hdwy - <		-	-	-	-	-	-
Critical Hdwy Stg 1 -		-	-	-	-	-	-
Critical Hdwy Stg 2 -		-	-	_	-	-	-
Follow-up Hdwy		-					-
Pot Cap-1 Maneuver 0 0 - - 0 Stage 1 0 0 0 - - 0 Stage 2 0 0 0 - - 0 Platoon blocked, % - - - - - 0 Mov Cap-1 Maneuver - - - - - - - Mov Cap-2 Maneuver - <td></td> <td>-</td> <td>_</td> <td>_</td> <td>_</td> <td>_</td> <td>_</td>		-	_	_	_	_	_
Stage 1 0 0 0 - - 0 Stage 2 0 0 0 - - 0 Platoon blocked, % -							0
Stage 2 0 0 0 - - 0 Platoon blocked, % -			~	-			0
Platoon blocked, % - - Mov Cap-1 Maneuver - - - - Mov Cap-2 Maneuver - - - - - - Stage 1 -		_					
Mov Cap-1 Maneuver -		U	U	U			0
Mov Cap-2 Maneuver -	•						
Stage 1 - </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>_</td>							_
Stage 2 - </td <td></td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>_</td> <td>-</td>		-	-	-	-	_	-
Approach EB NB SB HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBT EBLn1 EBLn2 SBT Capacity (veh/h) - - - HCM Lane V/C Ratio - - -		-	-	-	-	-	-
HCM Control Delay, s	Stage 2	-	-	-	-	-	-
HCM Control Delay, s							
HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBT EBLn1 EBLn2 SBT Capacity (veh/h) HCM Lane V/C Ratio	Approach	EB		NB		SB	
HCM LOS A Minor Lane/Major Mvmt NBT EBLn1 EBLn2 SBT Capacity (veh/h)		0		0		0	
Minor Lane/Major Mvmt NBT EBLn1 EBLn2 SBT Capacity (veh/h) HCM Lane V/C Ratio							
Capacity (veh/h) HCM Lane V/C Ratio							
Capacity (veh/h) HCM Lane V/C Ratio	NA:		NDT	-DI - 1	EDL - C	007	
HCM Lane V/C Ratio							
HCM Control Delay (s) - 0 0 -							
, , ,			-			-	
HCM Lane LOS - A A -			-	Α		-	
HCM 95th %tile Q(veh)	HCM 95th %tile Q(veh)		-	-	-	-	

Lane Configurations		•	•	4	†	ļ	1		
Traffic Volume (veh/h) 0 0 0 66 4116 1 Future Volume (Veh/h) 0 0 0 66 4116 1 Sign Control Stop Free Free Grade 0% 0% 0% 0% Peak Hour Factor 0.83 0.83 0.64 0.64 0.46 0.46 Hourly flow rate (vph) 0 0 0 103 8948 2 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type None None Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC, conflicting volume vC, stage 2 conf vol vCu, unblocked vol 8983 2238 8950 vCu, stage 2 conf vol vCu, unblocked vol 100 100 100 100 100 100 100 100 100 10	Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Traffic Volume (veh/h) 0 0 0 66 4116 1 Future Volume (Veh/h) 0 0 0 66 4116 1 Sign Control Stop Free Free Grade 0% 0% 0% 0% Peak Hour Factor 0.83 0.83 0.64 0.64 0.46 0.46 Hourly flow rate (vph) 0 0 0 103 8948 2 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type None None Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC3, stage 2 conf vol vC4, stage 1 conf vol vC5, stage 2 conf vol vC4, stage 1 conf vol vC5, stage 2 conf vol vC6, and stage (s) If (s) 3.5 3.3 2.2 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 34 2557 2557 2557 1280 Volume Left 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Left 0 0 0 0 0 0 0 0 0 Volume Left 0 0 0 0 0 0 0 0 0 Volume Left 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Left 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Left 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Lane Configurations				AAA	tttt.			
Future Volume (Veh/h) 0 0 0 666 4116 1 Sign Control Stop Free Free Grade 0% 0% 0% 0% 0% Peak Hour Factor 0.83 0.83 0.64 0.64 0.46 0.46 0.46 0.46 0.40 0.46 0.46		0	0	0			1		
Sign Control Stop									
Grade 0% 0% 0% 0% 0% 0% 0% 0 0 0 0 0 0 0 0 0							•		
Peak Hour Factor 0.83 0.83 0.64 0.64 0.46 0.	<u> </u>								
Hourly flow rate (vph)			0.83	0.64			0.46		
Pedestrians Lane Width (ft) Walking Speed (ft/s)									
Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type Median storage veh) Upstream signal (ft) px, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 8 6.8		U	U	U	103	0340			
Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median storage veh) Upstream signal (ft) Pystream signal (ft) Pystream signal (ft) VC2, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC3, stage 2 conf vol VC4, unblocked vol VC5, stage 2 conf vol VC6, single (s) VC7, 2 stage (s) VC7, 2 stage (s) VC8, 100 VC9, 100									
Percent Blockage Right turn flare (veh) Median type None None Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 8983 2238 8950 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage (s) E F F S S S S S S S S S S S S S S S S									
Right turn flare (veh) Median type Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC3, stage 3 conf vol vC4, single (s) tC, single (s) tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 34 35 Volume Right 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0									
Median type None None Median storage veh) Upstream signal (ft) VX, platoon unblocked vC, conflicting volume 8983 2238 8950 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 8983 2238 8950 tC, single (s) 6.8 6.9 4.1 6.8 6.9 4.1 tC, single (s) 3.5 3.3 2.2 5.2 5.2 5.3 5.2 5.3 5.3 5.2 5.3 5.3 5.2 5.3 5.3 5.2 5.3 5.3 5.2 5.3 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>									
Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 8983 2238 8950 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, unblocked vol 8983 2238 8950 tC, single (s) 6.8 6.9 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 2557 2557 2557 1280 Volume Left 0					None	None			
Upstream signal (ft) pX, platoon unblocked vC, conflicting volume					ivone	None			
pX, platoon unblocked vC, conflicting volume									
VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC3, stage 2 conf vol VC4, unblocked vol RC5, single (s) RC7, stage 1 conf vol RC7, stage 2 conf vol RC7, stage 1 conf vol RC7, stage 2 con RC7, sta									
vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 8983 2238 8950 tC, single (s) 6.8 6.9 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 2557 2557 2557 1280 Volume Right 0 0 0 0 0 0 0 0 cSH 1700 1700 1700 1700 1700 1700 Volume to Capacity 0.02 0.02 0.02 1.50 1.50 1.50 0.75 Queue Length 95th (ft) 0 0 0 0 0 0 0 0 Control Delay (s) 0.0 0.0 0.0 Approach LOS Intersection Summary Average Delay Intersection Capacity Utilization 8983 2238 8950 4.1 100 100 100 100 100 100 100 100 100 1		2000	0000	0050					
vC2, stage 2 conf vol vCu, unblocked vol 8983 2238 8950 tC, single (s) 6.8 6.9 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 2557 2557 2557 1280 Volume Right 0 0 0 0 0 0 0 0 cSH 1700 1700 1700 1700 1700 1700 1700 Volume to Capacity 0.02 0.02 0.02 1.50 1.50 1.50 0.75 Queue Length 95th (ft) 0 0 0 0 0 0 0 Control Delay (s) 0.0 0.0 0.0 0.0 Approach LoS Intersection Summary Average Delay Intersection Capacity Utilization Response Summary Intersection Capacity Utilization ICU Level of Service	vC, conflicting volume	8983	2238	8950					
vCu, unblocked vol 8983 2238 8950 tC, single (s) 6.8 6.9 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 2557 2557 2557 1280 Volume Left 0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>									
tC, single (s) 6.8 6.9 4.1 tC, 2 stage (s) tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 2557 2557 2557 1280 Volume Left 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 2 cSH 1700 1700 1700 1700 1700 1700 1700 170									
tC, 2 stage (s) tF (s)									
tF (s) 3.5 3.3 2.2 p0 queue free % 100 100 100 cM capacity (veh/h) 0 35 0 Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4 Volume Total 34 34 34 2557 2557 2557 1280 Volume Left 0 0 0 0 0 0 0 0 0 Volume Right 0 0 0 0 0 0 0 0 2 cSH 1700 1700 1700 1700 1700 1700 1700 Volume to Capacity 0.02 0.02 0.02 1.50 1.50 1.50 0.75 Queue Length 95th (ft) 0 0 0 0 0 0 0 0 Control Delay (s) 0.0 0.0 0.0 0.0 0.0 Approach Delay (s) 0.0 0.0 Intersection Summary Average Delay Intersection Capacity Utilization 63.0% ICU Level of Service		6.8	6.9	4.1					
p0 queue free % 100 100 100									
Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4									
Direction, Lane # NB 1 NB 2 NB 3 SB 1 SB 2 SB 3 SB 4									
Volume Total 34 34 34 2557 2557 2557 1280 Volume Left 0	cM capacity (veh/h)	0	35	0					
Volume Left 0 2 ccsH 1700 <t< td=""><td>Direction, Lane #</td><td>NB 1</td><td>NB 2</td><td>NB 3</td><td></td><td>SB 2</td><td></td><td>SB 4</td><td></td></t<>	Direction, Lane #	NB 1	NB 2	NB 3		SB 2		SB 4	
Volume Right 0 0 0 0 0 0 2 cSH 1700 1700 1700 1700 1700 1700 1700 Volume to Capacity 0.02 0.02 0.02 1.50 1.50 1.50 0.75 Queue Length 95th (ft) 0 0 0 0 0 0 0 Control Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Approach Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Intersection Summary 0.0 0.0 0.0 ICU Level of Service	Volume Total	34	34	34	2557	2557	2557	1280	
CSH 1700 1700 1700 1700 1700 1700 1700 170	Volume Left	0	0	0				0	
Volume to Capacity 0.02 0.02 0.02 1.50 1.50 0.75 Queue Length 95th (ft) 0 0 0 0 0 0 0 Control Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 Lane LOS Approach Delay (s) 0.0 0.0 0.0 0.0 Approach LOS Intersection Summary 0.0	Volume Right	0	0	0	0	0	0	2	
Volume to Capacity 0.02 0.02 0.02 1.50 1.50 0.75 Queue Length 95th (ft) 0 0 0 0 0 0 0 Control Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Lane LOS Approach Delay (s) 0.0	cSH	1700	1700	1700	1700	1700	1700	1700	
Queue Length 95th (ft) 0 <td>Volume to Capacity</td> <td>0.02</td> <td></td> <td>0.02</td> <td>1.50</td> <td></td> <td>1.50</td> <td>0.75</td> <td></td>	Volume to Capacity	0.02		0.02	1.50		1.50	0.75	
Control Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Lane LOS Approach Delay (s) 0.0 0.0 Approach LOS Intersection Summary Average Delay Intersection Capacity Utilization 63.0% ICU Level of Service	Queue Length 95th (ft)	0	0	0	0	0	0		
Lane LOS Approach Delay (s) 0.0 Approach LOS Intersection Summary Average Delay 0.0 Intersection Capacity Utilization 63.0% ICU Level of Service		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Approach LOS Intersection Summary Average Delay Intersection Capacity Utilization 63.0% ICU Level of Service	Lane LOS								
Approach LOS Intersection Summary Average Delay Intersection Capacity Utilization 63.0% ICU Level of Service	Approach Delay (s)	0.0			0.0				
Average Delay 0.0 Intersection Capacity Utilization 63.0% ICU Level of Service	Approach LOS								
Average Delay 0.0 Intersection Capacity Utilization 63.0% ICU Level of Service	Intersection Summary								
Intersection Capacity Utilization 63.0% ICU Level of Service				0.0					
		zation			IC	U Level o	f Service		
	Analysis Period (min)			15			. 5060		

Intersection												
Int Delay, s/veh	1.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	**		*	***	
Traffic Vol, veh/h	1	1	1	1	1	1	1	65	1	1	4116	1
Future Vol, veh/h	1	1	1	1	1	1	1	65	1	1	4116	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	25	25	25	50	50	50	67	67	67	79	79	79
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	4	4	4	2	2	2	1	97	1	1	5210	1
Major/Minor	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	5255	5313	2606	2188	5313	49	5211	0	0	98	0	0
Stage 1	5213	5213	_	100	100	_	_	-	_	-	-	-
Stage 2	42	100	_	2088	5213	_	_	_	_	_	_	_
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	_	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	_	_	-	_	_
Critical Hdwy Stg 2	6.72	5.52	_	6.72	5.52	-	_	-	_	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	0	0	16	49	0	858	2	_	_	1044	_	_
Stage 1	0	~ 2	-	813	814	-	-	-	-	-	-	-
Stage 2	893	814	_	48	2	-	-	-	-	-	-	-
Platoon blocked, %								-	-		_	_
Mov Cap-1 Maneuver	0	0	16	22	0	858	2	_	-	1044	-	-
Mov Cap-2 Maneuver	0	0	-	22	0	-	-	-	-	-	-	-
Stage 1	0	~ 2	-	407	407	-	-	-	-	-	-	-
Stage 2	443	407	-	-	2	-	_	_	_	-	_	_
3 10 3												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	\$ 455			101.9			37.7			0		
HCM LOS	F			F								
Minor Lane/Major Mvm	t	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		2	-	-	16	43	1044	-	-			
HCM Lane V/C Ratio		0.746	-	-	0.75	0.14	0.001	-	-			
HCM Control Delay (s)	\$	2527.5	-	-	\$ 455	101.9	8.5	-	-			
HCM Lane LOS	Ψ.	F	-	-	F	F	A	-	-			
HCM 95th %tile Q(veh)		0.7	-	-	1.9	0.4	0	-	_			
		- U.1				J. 1						
Notes		Φ. D.		- 1- 00	0-	0:	1-1'	L. D. C		k A II		
-: Volume exceeds cap	pacity	\$: Del	ay exce	eds 30	US +:	Compu	itation N	lot Defii	ned	: All ma	ajor volu	ıme ın p

Intersection												
Int Delay, s/veh	12.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43-		75	^ ^		7	444	
Traffic Vol, veh/h	1	1	8	14	1	1	1	64	23	7	4111	1
Future Vol, veh/h	1	1	8	14	1	1	1	64	23	7	4111	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	300	-	-	300	-	-
Veh in Median Storage,	, # -	0	-	-	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	40	40	40	54	54	54	80	80	80	81	81	81
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	20	26	2	2	1	80	29	9	5075	1
Major/Minor N	Minor2			Minor1			Major1		ı	Major2		
Conflicting Flow All	5129	5205	2538	2147	5191	55	5076	0	0	109	0	0
Stage 1	5094	5094	2000	97	97	- 35	5076	-	-	109	-	-
Stage 1	35	111	_	2050	5094	-	-	-	_	-	-	
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	_	-
•			7.12		5.52	7.12	5.52	_	_	0.32	_	
Critical Hdwy Stg 1	7.32 6.72	5.52 5.52	_	7.32 6.72	5.52		_	-	-	_	-	_
Critical Hdwy Stg 2	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	_	3.11		
Follow-up Hdwy	ا م. 2 م				4.01		3.11	-	-		-	_
Pot Cap-1 Maneuver	~ 1	0 ~ 2	~ 18	52 816		851	3	-	-	1032	-	-
Stage 1	-		-		816 2		-	-	-	_		-
Stage 2	902	805	-	50	2	-	-	-	-	-		-
Platoon blocked, %		•	40		0	054	0	-	-	4000	-	-
Mov Cap-1 Maneuver	~ 1	0	~ 18	-	0	851	3	-	-	1032	-	-
Mov Cap-2 Maneuver	~ 1	0	-	-	0	-	-	-	-	-	-	-
Stage 1	0	~ 2	-	544	544	-	-	-	-	-	-	-
Stage 2	598	537	-	~ 1	2	-	-	-	_	-	-	_
Approach	EB			WB			NB			SB		
HCM Control Delay, \$ 2							17.8			0		
HCM LOS	F			-								
Minor Lane/Major Mvmt	t	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		3	_	_	6	-	1032	-	-			
HCM Lane V/C Ratio		0.417	-	_	4.167	-	0.008	-	-			
HCM Control Delay (s)	\$	1565.9	_		2595.3	_	8.5	_	_			
HCM Lane LOS	Ψ	F	_	Ψ.	F	_	Α	_	_			
HCM 95th %tile Q(veh)		0.6	_	_	4.5	_	0	_	_			
		0.0			7.5		J					
Notes												
~: Volume exceeds cap	acity	\$: Del	ay exce	eds 300)s +:	Compu	Itation N	lot Defii	ned '	: All ma	ajor volu	ıme in p

Appendix G: SimTraffic Report – 2025 Build Conditions



Intersection: 6: US 59 & Existing Entrance 2

Movement	EB
Directions Served	R
Maximum Queue (ft)	12
Average Queue (ft)	1
95th Queue (ft)	7
Link Distance (ft)	1028
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 8: US 59 & Entrance to Webb County Road and Bridges/Wilson Rd

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	L	L
Maximum Queue (ft)	33	35	2	5
Average Queue (ft)	4	6	0	0
95th Queue (ft)	20	25	2	5
Link Distance (ft)	1056	40		
Upstream Blk Time (%)		0		
Queuing Penalty (veh)		0		
Storage Bay Dist (ft)			300	300
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 12: US 59 & State Rep. Henry Cuellar Rdwy/Heritage Ct

Movement	EB	WB	NB	SB	SB	
Directions Served	LTR	LTR	L	L	Т	
Maximum Queue (ft)	38	124	26	7	2	
Average Queue (ft)	7	46	4	0	0	
95th Queue (ft)	29	106	19	4	2	
Link Distance (ft)	980	198			653	
Upstream Blk Time (%)		0				
Queuing Penalty (veh)		0				
Storage Bay Dist (ft)			300	300		
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 15: US 59 & Proposed Main Entrance

NB	NB	SB	SB
L	L	T	R
172	181	195	75
99	111	84	36
148	167	159	63
652	652	323	323
	L 172 99 148	L L 172 181 99 111 148 167	L L T 172 181 195 99 111 84 148 167 159

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

Intersection: 17: US 59 & Proposed Entrance 2

Movement	EB	EB
Directions Served	R	R
Maximum Queue (ft)	97	39
Average Queue (ft)	4	2
95th Queue (ft)	42	29
Link Distance (ft)	1112	1112
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 19: US 59 & Proposed Entrance 3

Movement	NB	NB	SB	SB	SB
Directions Served	L	L	Т	T	TR
Maximum Queue (ft)	152	164	4	4	4
Average Queue (ft)	62	68	0	0	0
95th Queue (ft)	120	129	4	3	3
Link Distance (ft)	449	449	417	417	417
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Zone Summary

Intersection: 6: US 59 & Existing Entrance 2

Movement	SB	SB	SB	SB
Directions Served	T	T	Т	TR
Maximum Queue (ft)	62	247	288	260
Average Queue (ft)	15	137	177	172
95th Queue (ft)	88	283	314	295
Link Distance (ft)	451	451	451	451
Upstream Blk Time (%)				

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

Intersection: 8: US 59 & Entrance to Webb County Road and Bridges/Wilson Rd

Movement	EB	WB	NB	SB	
Directions Served	LTR	LTR	L	TR	
Maximum Queue (ft)	15	13	3	2	
Average Queue (ft)	4	5	1	0	
95th Queue (ft)	20	24	6	4	
Link Distance (ft)	1056	40		717	
Upstream Blk Time (%)		10			
Queuing Penalty (veh)		0			
Storage Bay Dist (ft)			300		
Storage Blk Time (%)					
Queuing Penalty (veh)					

Quoding i charty (voii)

Intersection: 12: US 59 & State Rep. Henry Cuellar Rdwy/Heritage Ct

Movement	EB	WB	NB	SB	SB	
Directions Served	LTR	LTR	L	L	Т	
Maximum Queue (ft)	53	98	2	3	4	
Average Queue (ft)	27	48	2	0	1	
95th Queue (ft)	62	106	10	5	8	
Link Distance (ft)	980	198			653	
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			300	300		
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 15: US 59 & Proposed Main Entrance

Movement	EB	EB	NB	NB	SB
Directions Served	L	R	L	T	Т
Maximum Queue (ft)	451	244	32	51	59
Average Queue (ft)	196	38	14	14	27
95th Queue (ft)	489	351	38	46	67
Link Distance (ft)	1128	1128	647	647	355
Upstream Blk Time (%)	0	0			
Queuing Penalty (veh)	0	0			
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 17: US 59 & Proposed Entrance 2

EB
R
9
2
13
1101

Intersection: 19: US 59 & Proposed Entrance 3

Movement	EB	EB
Directions Served	R	R
Maximum Queue (ft)	498	478
Average Queue (ft)	386	364
95th Queue (ft)	495	477
Link Distance (ft)	1064	1064
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Zone Summary

Appendix H: Synchro Report – 2035 Build Conditions



	•	•	~	†	Ţ	4			
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations			ሻሻ	†	+	7			
Traffic Volume (vph)	0	0	563	186	294	113			
Future Volume (vph)	0	0	563	186	294	113			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Total Lost time (s)			4.0	4.0	4.0	4.0			
Lane Util. Factor			0.97	1.00	1.00	1.00			
Frt			1.00	1.00	1.00	0.85			
Flt Protected			0.95	1.00	1.00	1.00			
Satd. Flow (prot)			3467	1727	1727	1599			
FIt Permitted			0.95	1.00	1.00	1.00			
Satd. Flow (perm)			3467	1727	1727	1599			
Peak-hour factor, PHF	0.93	0.93	0.82	0.82	0.82	0.82			
Adj. Flow (vph)	0	0	687	227	359	138			
RTOR Reduction (vph)	0	0	0	0	0	101			
Lane Group Flow (vph)	0	0	687	227	359	37			
Heavy Vehicles (%)	1%	1%	1%	10%	10%	1%			
Turn Type			Prot	NA	NA	Perm			
Protected Phases			7	468	8				
Permitted Phases						8			
Actuated Green, G (s)			16.0	60.0	16.0	16.0			
Effective Green, g (s)			16.0	60.0	16.0	16.0			
Actuated g/C Ratio			0.27	1.00	0.27	0.27			
Clearance Time (s)			4.0		4.0	4.0			
Lane Grp Cap (vph)			924	1727	460	426			
v/s Ratio Prot			c0.20	c0.13	c0.21				
v/s Ratio Perm						0.02			
v/c Ratio			0.74	0.13	0.78	0.09			
Uniform Delay, d1			20.1	0.0	20.4	16.5			
Progression Factor			1.00	1.00	1.00	1.00			
Incremental Delay, d2			4.7	0.1	12.4	0.4			
Delay (s)			24.8	0.1	32.7	16.9			
Level of Service			С	Α	С	В			
Approach Delay (s)	0.0			18.7	28.3				
Approach LOS	Α			В	С				
Intersection Summary									
HCM 2000 Control Delay			22.1	Н	CM 2000	Level of Service	9	С	
HCM 2000 Volume to Capac	city ratio		0.56						
Actuated Cycle Length (s)			60.0	S	um of lost	time (s)		12.0	
Intersection Capacity Utilizat	tion		38.2%			of Service		Α	
Analysis Period (min)			15						

Analysis Period (min)
c Critical Lane Group

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL	T T	TIDL	^	<u>180</u>	OBIN
Traffic Vol, veh/h	0	1	0	749	294	1
Future Vol, veh/h	0	1	0	749	294	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	_	0	_	-	_	-
Veh in Median Storage		-	_	0	0	_
Grade, %	0	_	_	0	0	<u>-</u>
Peak Hour Factor	62	62	82	82	82	82
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	0	2	0	913	359	1
IVIVIIIL I IOW	U		U	313	333	
Major/Minor N	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	-	360	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.215	-	-	-	-
Critical Hdwy Stg 1	_	-	_	-	_	-
Critical Hdwy Stg 2	_	-	_	_	_	-
Follow-up Hdwy	- (3.3095	-	_	_	_
Pot Cap-1 Maneuver	0	686	0	_	-	-
Stage 1	0	-	0	_	_	_
Stage 2	0	_	0	_	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	_	686	_	_	_	_
Mov Cap-1 Maneuver		-	_	_	_	_
Stage 1	-		-	-		-
•	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	10.3		0		0	
HCM LOS	В					
113.117 200						
Minor Lane/Major Mvm	t	NBT E	EBLn1	SBT	SBR	
Capacity (veh/h)		-		-	-	
HCM Lane V/C Ratio		-	0.002	-	-	
HCM Control Delay (s)		-	10.3	-	-	
HCM Lane LOS		-	В	-	-	
HCM 95th %tile Q(veh)		-	0	-	-	

Intersection						
Int Delay, s/veh	0					
		===			0==	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		7		^	<u></u>	
Traffic Vol, veh/h	0	455	0	749	294	0
Future Vol, veh/h	0	455	0	749	294	0
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Free	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	62	62	82	82	82	82
Heavy Vehicles, %	1	1	1	10	10	1
Mvmt Flow	0	734	0	913	359	0
Major/Minor M	inor2	N	/lajor1		/lajor2	
						0
Conflicting Flow All	-	-	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-
Pot Cap-1 Maneuver	0	0	0	-	-	0
Stage 1	0	0	0	-	-	0
Stage 2	0	0	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	-	-	-	_	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	_	-
Stage 2	-	-	-	-	-	-
J. Control of the con						
			ND		0.0	
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NRT F	EBLn1	SBT		
Capacity (veh/h)		NOTE	LULIII	-		
HCM Lane V/C Ratio		-	_	-		
HCM Control Delay (s)		_	0			
HCM Lane LOS		-	A	-		
HCM 95th %tile Q(veh)						
HOW South Malle Q(ven)		-	-	-		

	۶	\rightarrow		†	ļ	4			
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations			ሻሻ	†	^				
Traffic Volume (vph)	0	0	450	749	749	0			
Future Volume (vph)	0	0	450	749	749	0			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Total Lost time (s)			4.0	4.0	4.0				
Lane Util. Factor			0.97	1.00	0.95				
Frt			1.00	1.00	1.00				
Flt Protected			0.95	1.00	1.00				
Satd. Flow (prot)			3467	1727	3282				
FIt Permitted			0.95	1.00	1.00				
Satd. Flow (perm)			3467	1727	3282				
Peak-hour factor, PHF	0.93	0.93	0.82	0.82	0.82	0.82			
Adj. Flow (vph)	0	0	549	913	913	0			
RTOR Reduction (vph)	0	0	0	0	0	0			
Lane Group Flow (vph)	0	0	549	913	913	0			
Heavy Vehicles (%)	1%	1%	1%	10%	10%	1%			
Turn Type			Prot	NA	NA				
Protected Phases			7	468	8				
Permitted Phases			•		-				
Actuated Green, G (s)			11.0	60.0	20.0				
Effective Green, g (s)			11.0	60.0	20.0				
Actuated g/C Ratio			0.18	1.00	0.33				
Clearance Time (s)			4.0		4.0				
Lane Grp Cap (vph)			635	1727	1094				
v/s Ratio Prot			c0.16	c0.53	c0.28				
v/s Ratio Perm									
v/c Ratio			0.86	0.53	0.83				
Uniform Delay, d1			23.8	0.0	18.5				
Progression Factor			1.00	1.00	1.33				
Incremental Delay, d2			14.6	1.2	7.4				
Delay (s)			38.4	1.2	31.9				
Level of Service			D	Α	С				
Approach Delay (s)	0.0			15.1	31.9				
Approach LOS	Α			В	С				
Intersection Summary									
HCM 2000 Control Delay			21.6	Н	CM 2000	Level of Service		С	
HCM 2000 Volume to Capac	ity ratio		0.78						
Actuated Cycle Length (s)			60.0	S	um of lost	time (s)	_ 1:	2.0	
Intersection Capacity Utilizat	ion		42.8%		CU Level c			A	
Analysis Period (min)			15						

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		*	ተተተ		ች	ተተተ	
Traffic Vol, veh/h	1	1	1	5	1	5	1	1193	9	1	749	1
Future Vol., veh/h	1	1	1	5	1	5	1	1193	9	1	749	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	38	38	38	75	75	75	91	91	91	89	89	89
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	3	7	1	7	1	1311	10	1	842	1
Major/Minor N	/linor2		N	Minor1			Major1		N	//ajor2		
Conflicting Flow All	1372	2168	422	1658	2163	661	843	0	0	1321	0	0
Stage 1	845	845	-	1318	1318	-	-	-	-	-	-	-
Stage 2	527	1323	-	340	845	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	155	47	498	104	47	349	469	-	-	276	_	-
Stage 1	256	379	-	120	227	-	-	-	-	-	-	-
Stage 2	461	226	-	596	379	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	148	47	498	99	47	349	469	-	-	276	-	-
Mov Cap-2 Maneuver	148	47	-	99	47	-	-	-	-	-	-	-
Stage 1	255	377	-	120	227	-	-	-	-	-	-	-
Stage 2	449	226	-	587	377	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	44.1			36.7			0			0		
HCM LOS	Е			Е								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		469	-	-		128	276	-	-			
HCM Lane V/C Ratio		0.002	_	_		0.115		_	_			
HCM Control Delay (s)		12.7	-	-	44.1	36.7	18.1	_	-			
HCM Lane LOS		В	_	_	E	E	C	_	_			
HCM 95th %tile Q(veh)		0	-	-	0.3	0.4	0	-	-			

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	^		ነ	ተተተ	
Traffic Vol, veh/h	1	1	1	73	1	7	12	1193	92	2	745	7
Future Vol, veh/h	1	1	1	73	1	7	12	1193	92	2	745	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	33	33	33	86	86	86	92	92	92	91	91	91
Heavy Vehicles, %	1	1	1	1	1	1	1	10	1	1	10	1
Mvmt Flow	3	3	3	85	1	8	13	1297	100	2	819	8
Major/Minor N	Minor2		<u> </u>	Minor1			Major1		<u> </u>	Major2		
Conflicting Flow All	1372	2250	414	1706	2204	699	827	0	0	1397	0	0
Stage 1	827	827	-	1373	1373	_	-	-	-	-	-	-
Stage 2	545	1423	-	333	831	-	-	-	-	-	-	-
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.32	-	-	5.32	-	-
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.11	-	-	3.11	-	-
Pot Cap-1 Maneuver	155	42	504	97	45	330	477	-	-	253	-	-
Stage 1	264	387	-	110	214	-	-	-	-	-	-	-
Stage 2	450	202	-	602	385	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	144	41	504	89	43	330	477	-	-	253	-	-
Mov Cap-2 Maneuver	144	41	-	89	43	-	-	-	-	-	-	-
Stage 1	257	384	-	107	208	-	-	-	-	-	-	-
Stage 2	425	197	-	589	382	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	49.4			175.2			0.1			0.1		
HCM LOS	Е			F								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		477	_	_	90	94	253	_	_			
HCM Lane V/C Ratio		0.027	-	-	0.101	1.002		-	-			
HCM Control Delay (s)		12.8	-	-		175.2	19.4	-	-			
HCM Lane LOS		В	-	-	Е	F	С	-	-			
HCM 95th %tile Q(veh)		0.1	-	-	0.3	6	0	-	-			

	•	•	4	†	ļ	1			
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	ች	77	ች	^		7			
Traffic Volume (vph)	455	1591	24	95	39	1			
Future Volume (vph)	455	1591	24	95	39	1			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0			
Lane Util. Factor	1.00	0.88	1.00	0.95	1.00	1.00			
Frt	1.00	0.85	1.00	1.00	1.00	0.85			
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00			
Satd. Flow (prot)	1805	2842	1787	3574	1881	1615			
FIt Permitted	0.95	1.00	0.57	1.00	1.00	1.00			
Satd. Flow (perm)	1805	2842	1079	3574	1881	1615			
Peak-hour factor, PHF	0.83	0.83	0.64	0.64	0.64	0.64			
Adj. Flow (vph)	548	1917	38	148	61	2			
RTOR Reduction (vph)	0	0	0	0	0	0			
Lane Group Flow (vph)	548	1917	38	148	61	2			
Heavy Vehicles (%)	0%	0%	1%	1%	1%	0%			
Turn Type	Prot	Free		NA	NA	Free			
Protected Phases	5		7	468	8				
Permitted Phases		Free	4			Free			
Actuated Green, G (s)	47.0	100.0	24.0	45.0	16.0	100.0			
Effective Green, g (s)	47.0	100.0	24.0	45.0	16.0	100.0			
Actuated g/C Ratio	0.47	1.00	0.24	0.45	0.16	1.00			
Clearance Time (s)	4.0		4.0		4.0				
Lane Grp Cap (vph)	848	2842	287	1608	300	1615			
v/s Ratio Prot	0.30		0.01	0.04	0.03	1010			
v/s Ratio Perm		c0.67	0.03			0.00			
v/c Ratio	0.65	0.67	0.13	0.09	0.20	0.00			
Uniform Delay, d1	20.2	0.0	29.6	15.8	36.5	0.0			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	3.8	1.3	1.0	0.1	1.5	0.0			
Delay (s)	24.0	1.3	30.5	15.9	38.0	0.0			
Level of Service	С	Α	С	В	D	А			
Approach Delay (s)	6.3			18.9	36.8				
Approach LOS	Α			В	D				
Intersection Summary									
HCM 2000 Control Delay			7.9	ш	CM 2000	Level of Service	` <u>`</u>	A	
HCM 2000 Control Delay HCM 2000 Volume to Capa	city ratio		0.80	П	CIVI ZUUU	Level Of Service	00	A	
Actuated Cycle Length (s)	city ratio		100.0	Q	um of lost	time (s)		16.0	
Intersection Capacity Utiliza	ation		39.9%			of Service		A	
Analysis Period (min)	iuOH		15	IC.	O FEARI	DI OGIVICE		^	
Alialysis Fellou (IIIIII)			13						

Analysis Period (min)
c Critical Lane Group

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL	EDK	INDL			אפט
Traffic Vol, veh/h	0		0	TTT 120	↑↑३	1
Future Vol, veh/h	0	1	0	120	1630	1
Conflicting Peds, #/hr	0	0	0	0	0	0
			Free	Free	Free	Free
Sign Control RT Channelized	Stop -	Stop None	Free -		Free -	None
Storage Length	-	0	-	None -	_	None -
Veh in Median Storage		-	-	0	0	-
Grade, %	e, # 0 0		-	0	0	
	36	-	64	64	64	64
Peak Hour Factor		36				
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	3	0	188	2547	2
Major/Minor	Minor2	N	//ajor1	ı	Major2	
Conflicting Flow All	_	1275		0		0
Stage 1	_	-	_	-	_	-
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	7.1	_	_	_	_
Critical Hdwy Stg 1	_	-	_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.9	_	_	_	_
Pot Cap-1 Maneuver	0	138	0	_	_	_
Stage 1	0	-	0	_	_	_
Stage 2	0	_	0	_	_	_
Platoon blocked, %	U		U	_	_	_
Mov Cap-1 Maneuver	_	138	_	_	_	
Mov Cap-1 Maneuver	_	130	_	_	_	_
	-	-	-	-		-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			0		0	
HCM LOS	D					
				0	05-	
Minor Lane/Major Mvr	nt		EBLn1	SBT	SBR	
Capacity (veh/h)		-		-	-	
HCM Lane V/C Ratio		-	0.02	-	-	
HCM Control Delay (s)	-	31.6	-	-	
HCM Lane LOS		-	D	-	-	
HCM 95th %tile Q(veh	1)	-	0.1	-	-	

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	LDL	ZDK ř	NDL			אומט
	٥		٥	↑↑↑	↑ ↑ 1630	0
Traffic Vol, veh/h	0	909	0	120		0
Future Vol, veh/h	0	909	0	120	1630	0
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Free	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	36	36	64	64	64	64
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	2525	0	188	2547	0
WWITETIOW	U	2020	U	100	2041	U
Major/Minor I	Minor2	N	Major1	N	Major2	
Conflicting Flow All	-	-	-	0	-	0
Stage 1	-	-	-	-	-	_
Stage 2	_	-	-	_	_	-
Critical Hdwy	_	_	_	_	_	_
Critical Hdwy Stg 1	_	_	_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	_	_	_	_	_
Pot Cap-1 Maneuver	0	0	0	-	-	0
Stage 1	0	0	0	-	-	0
Stage 2	0	0	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	-	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	_	-	-	-	_	-
5 13 gc =						
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT E	EBLn1	SBT		
Capacity (veh/h)		-	-	-		
HCM Lane V/C Ratio		-	-	-		
HCM Control Delay (s)		-	0	-		
HCM Lane LOS		-	Α	-		
HCM 95th %tile Q(veh)		_	-	-		

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	LDL		NDL			
Lane Configurations	0	1501	٥	↑ ↑↑	^	7
Traffic Vol, veh/h	0	1591	0	120	2539	1
Future Vol, veh/h	0	1591	0	120	2539	1
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	Free	-	None	-	None
Storage Length	-	0	-	-	-	0
Veh in Median Storage	,#0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	83	83	64	64	46	46
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	1917	0	188	5520	2
N.A. '. (N.A'						
	Minor2	N	Major1		Major2	
Conflicting Flow All	-	-	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	_	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-
Pot Cap-1 Maneuver	0	0	0	_	-	_
Stage 1	0	0	0	_	_	_
Stage 2	0	0	0	_	_	_
Platoon blocked, %	U	U	U	_	_	_
Mov Cap-1 Maneuver	_	_	_		_	_
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		- 0			
TIOWI LOO						
Minor Lane/Major Mvm	t	NBT E	EBLn1	SBT	SBR	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		-	-	-	-	
HCM Control Delay (s)		-	0	-	-	
HCM Lane LOS		-	A	-	-	
HCM 95th %tile Q(veh)		_	-	_	_	

Intersection													
Int Delay, s/veh	2.7												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			ተተተ		*	ተተተ		
Traffic Vol, veh/h	1	1	2	2	1	2	1	118	2	1	4129	1	
Future Vol, veh/h	1	1	2	2	1	2	1	118	2	1	4129	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-	
/eh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	25	25	25	50	50	50	67	67	67	79	79	79	
Heavy Vehicles, %	1	1	1	1	1	1	0	1	0	0	1	0	
Mvmt Flow	4	4	8	4	2	4	1	176	3	1	5227	1	
Major/Minor N	/linor2		ľ	Minor1		ı	Major1		N	//ajor2			
Conflicting Flow All	5303	5411	2614	2275	5410	90	5228	0	0	179	0	0	
Stage 1	5230	5230	-	180	180	-	-	-	-	-	-	-	
Stage 2	73	181	-	2095	5230	-	-	-	_	-	-	-	
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.32	5.52	-	7.32	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52	-	-	-	-	-	-	-	
ollow-up Hdwy	3.81	4.01	3.91	3.81	4.01	3.91	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	0	0	16	43	0	809	2	-	-	963	-	-	
Stage 1	0	~ 2	-	720	752	-	-	-	-	-	-	-	
Stage 2	857	751	-	47	2	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	0	0	16	13	0	809	2	-	-	963	-	-	
Mov Cap-2 Maneuver	0	0	-	13	0	-	-	-	-	-	-	-	
Stage 1	0	~ 2	-	360	376	-	-	-	-	-	-	-	
Stage 2	424	376	-	-	2	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s\$	548.2			212.7			20.9			0			
HCM LOS	F			F									
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR				
Capacity (veh/h)		2	-	-	16	26	963	-	-				
HCM Lane V/C Ratio		0.746	-	_	1	0.385		_	-				
HCM Control Delay (s)	\$ 2	2527.5	-	-\$	548.2		8.7	-	-				
HCM Lane LOS		F	-	-	F	F	Α	-	-				
HCM 95th %tile Q(veh)		0.7	-	-	2.4	1.2	0	-	-				
Notes													
~: Volume exceeds cap	acity	\$: De	lay exc	eeds 30)0s -	+: Comp	outation	Not De	efined	*: All :	maior v	olume in	n platoon
ciamo oxocodo oup	Long	ψ. υ		2000 00		. 00111	Jacation	. 101 00		. 7 111	ajoi v	CIGITIO III	. platoon

2035 Conditions

Intersection													
Int Delay, s/veh	30.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4		7	^		7	^		
Traffic Vol, veh/h	2	1	12	21	1	2	1	116	35	11	4122	1	
Future Vol, veh/h	2	1	12	21	1	2	1	116	35	11	4122	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	40	40	40	54	54	54	80	80	80	81	81	81	
Heavy Vehicles, %	1	1	1	1	1	1	0	1	0	0	1	0	
Mvmt Flow	5	3	30	39	2	4	1	145	44	14	5089	1	
					_	•	•			• •		•	
Major/Minor I	Minor2		-	Minor1		ľ	Major1		N	//ajor2			
Conflicting Flow All	5179	5309	2545	2234	5287	95	5090	0	0	189	0	0	
Stage 1	5118	5118	-	169	169	-	-	-	-	-	-	-	
Stage 2	61	191	_	2065	5118	_	_	_	_	_		_	
Critical Hdwy	6.42	6.52	7.12	6.42	6.52	7.12	5.3	_		5.3	_		
	7.32	5.52	7.12	7.32	5.52	7.12	5.5	_	_	J.J -		_	
Critical Hdwy Stg 1 Critical Hdwy Stg 2	6.72	5.52	-	6.72	5.52		_			-	-	-	
, ,		4.01				2.01	3.1	-	-	2 1	-	-	
Follow-up Hdwy	3.81		3.91	3.81	4.01	3.91		-	-	3.1	-	-	
Pot Cap-1 Maneuver	~ 1	0	~ 18	46	0	803	3	-	-	953	-	-	
Stage 1	0	~ 2	-	732	760	-	-	-	-	-	-	-	
Stage 2	871	744	-	49	2	-	-	-	-	-	-	-	
Platoon blocked, %	4	^	40		^	000	•	-	-	050	-	-	
Mov Cap-1 Maneuver	~ 1	0	~ 18	-	0	803	3	-	-	953	-	-	
Mov Cap-2 Maneuver	~ 1	0	-	400	0	-	-	-	-	-	-	-	
Stage 1	0	~ 2	-	488	507	-	-	-	-	-	-	-	
Stage 2	576	496	-	~ 8	2	-	-	-	-	-	-	-	
A				1610			N.D.			0.0			
Approach	EB			WB			NB			SB			
HCM Control Delay, s S							10.3			0			
HCM LOS	F			-									
Minor Lane/Major Mvm	ıt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR				
Capacity (veh/h)		3	-	-	5	-	953	-	-				
HCM Lane V/C Ratio		0.417	-	-	7.5	-	0.014	-	-				
HCM Control Delay (s)	\$ '	1565.9	-	- 9	4325	-	8.8	-	-				
HCM Lane LOS		F	-	-	F	-	Α	-	-				
HCM 95th %tile Q(veh)		0.6	-	-	6.3	-	0	-	-				
Notes													
~: Volume exceeds car	pacity	\$· De	lav exc	eeds 30)0s -	+: Comp	outation	Not De	fined	*: All :	maior v	olume in	n platoon
Jiamo okooodo oap	Jaoney	Ψ. D0	y ono	5545 00		. 50111				. 7 111 1		CIGITIO III	. p.atoon

Appendix I: SimTraffic Report – 2035 Build Conditions



Intersection: 1: US 59 & Proposed Main Entrance

Movement	NB	NB	SB	SB
Directions Served	L	L	Т	R
Maximum Queue (ft)	128	152	201	50
Average Queue (ft)	94	108	124	32
95th Queue (ft)	133	156	203	54
Link Distance (ft)	655	655	332	332
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 2: US 59 & Proposed Entrance 2

Movement	EB
Directions Served	R
Maximum Queue (ft)	3
Average Queue (ft)	0
95th Queue (ft)	6
Link Distance (ft)	1117
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 3: US 59 & Proposed Entrance 3

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 4: US 59 & Existing Entrance 2

Movement	NB	NB	SB	SB
Directions Served	L	L	T	T
Maximum Queue (ft)	129	138	205	211
Average Queue (ft)	90	92	149	152
95th Queue (ft)	133	141	219	216
Link Distance (ft)	727	727	426	426
Upstream Blk Time (%)				
Queuing Penalty (veh)				
0. 5 5. (0)				

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

Intersection: 5: US 59 & Entrance to Webb County Road and Bridges/Wilson Rd

Movement	EB	WB	NB	SB	
Directions Served	LTR	LTR	L	L	
Maximum Queue (ft)	8	25	2	4	
Average Queue (ft)	1	6	0	1	
95th Queue (ft)	10	24	3	7	
Link Distance (ft)	1055	41			
Upstream Blk Time (%)		0			
Queuing Penalty (veh)		0			
Storage Bay Dist (ft)			100	100	
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 6: US 59 & State Rep. Henry Cuellar Rdwy/Heritage Ct

Movement	EB	WB	B14	NB	NB	SB
Directions Served	LTR	LTR	T	L	TR	L
Maximum Queue (ft)	18	135	6	17	7	10
Average Queue (ft)	4	73	2	3	1	1
95th Queue (ft)	21	173	18	14	9	10
Link Distance (ft)	976	217	797		1855	
Upstream Blk Time (%)		4				
Queuing Penalty (veh)		0				
Storage Bay Dist (ft)				100		100
Storage Blk Time (%)						
Queuing Penalty (veh)						

Zone Summary

Intersection: 1: US 59 & Proposed Main Entrance

Movement	EB	EB	EB	NB	NB	NB	SB
Directions Served	L	R	R	L	T	Т	T
Maximum Queue (ft)	532	691	5	37	38	50	55
Average Queue (ft)	199	135	1	16	15	25	30
95th Queue (ft)	540	721	10	41	43	57	66
Link Distance (ft)	1124	1124	1124	656	656	656	356
Upstream Blk Time (%)	0	1					
Queuing Penalty (veh)	0	0					
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 2: US 59 & Proposed Entrance 2

Movement	EB
Directions Served	R
Maximum Queue (ft)	3
Average Queue (ft)	0
95th Queue (ft)	6
Link Distance (ft)	1092
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 3: US 59 & Proposed Entrance 3

Movement	EB		
Directions Served	R		
Maximum Queue (ft)	11		
Average Queue (ft)	2		
95th Queue (ft)	23		
Link Distance (ft)	1068		
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 4: US 59 & Existing Entrance 2

Movement	SB	SB	SB
Directions Served	T	Т	R
Maximum Queue (ft)	78	87	48
Average Queue (ft)	6	7	7
95th Queue (ft)	85	96	100
Link Distance (ft)	431	431	431
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		2	1
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 5: US 59 & Entrance to Webb County Road and Bridges/Wilson Rd

Movement	EB	WB	NB	SB	B21
Directions Served	LTR	LTR	L	L	T
Maximum Queue (ft)	15	33	3	4	4
Average Queue (ft)	6	11	1	1	1
95th Queue (ft)	24	35	7	6	9
Link Distance (ft)	1055	41			270
Upstream Blk Time (%)		11			
Queuing Penalty (veh)		0			
Storage Bay Dist (ft)			100	100	
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 6: US 59 & State Rep. Henry Cuellar Rdwy/Heritage Ct

Movement	EB	WB	NB	SB	
Directions Served	LTR	LTR	L	L	
Maximum Queue (ft)	97	139	5	10	
Average Queue (ft)	49	84	2	2	
95th Queue (ft)	118	170	10	12	
Link Distance (ft)	979	566			
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)			100	100	
Storage Blk Time (%)					
Queuing Penalty (veh)					

Zone Summary